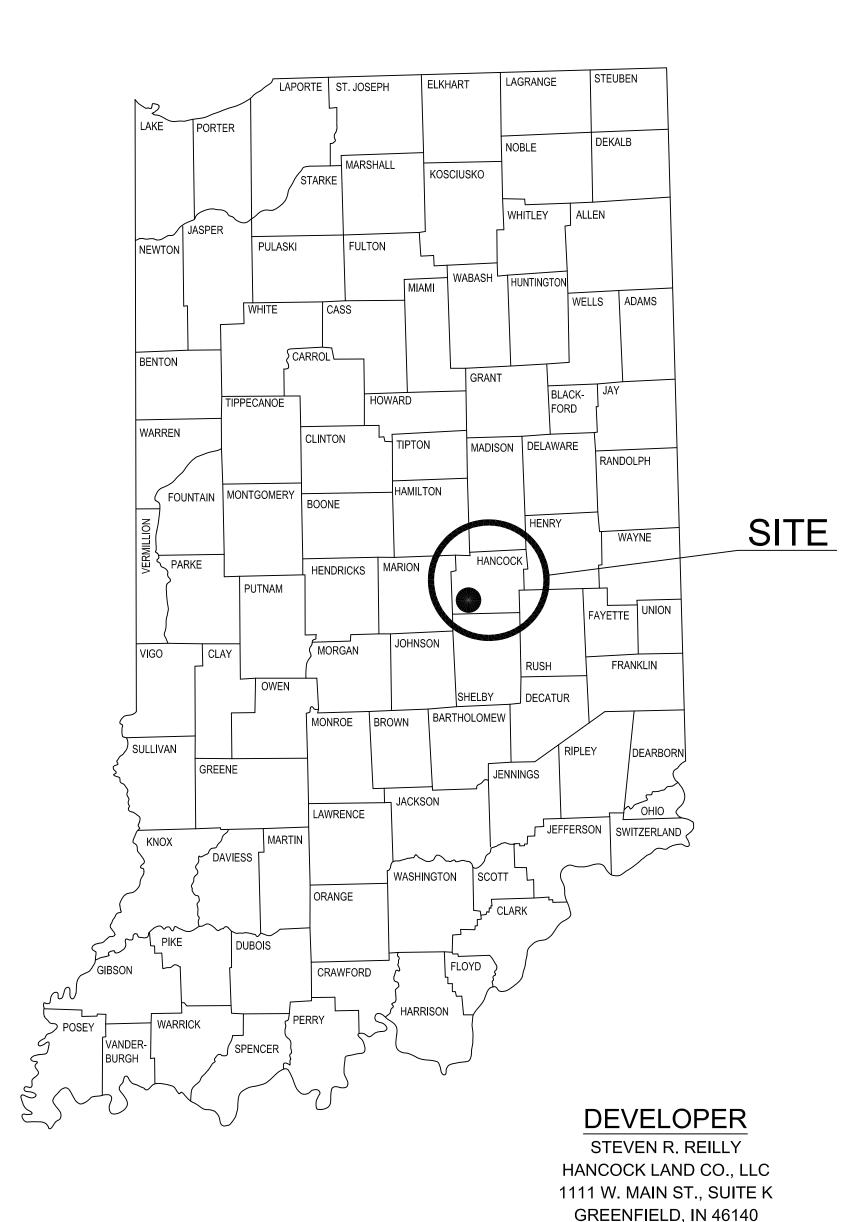
Site Development Plans For: "The Overlook Section Three" Section 20, Township 16 North, Range 6 East Sugar Creek Township, Hancock County, IN September, 2018





SHEET INDEX			
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Title Sheet	C1.0		
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NOTES: THESE DOCUMENTS, PLANS, AND SPECIFICATIONS ARE SUBJECT TO PERIODIC UPDATES AND REVISIONS. THE CONTRACTOR, OWNER, AND SUPPLIERS ARE RESPONSIBLE FOR VERIFICATION OF THE MOST CURRENT ISSUE AND REVISION, PRIOR TO ANY USE.

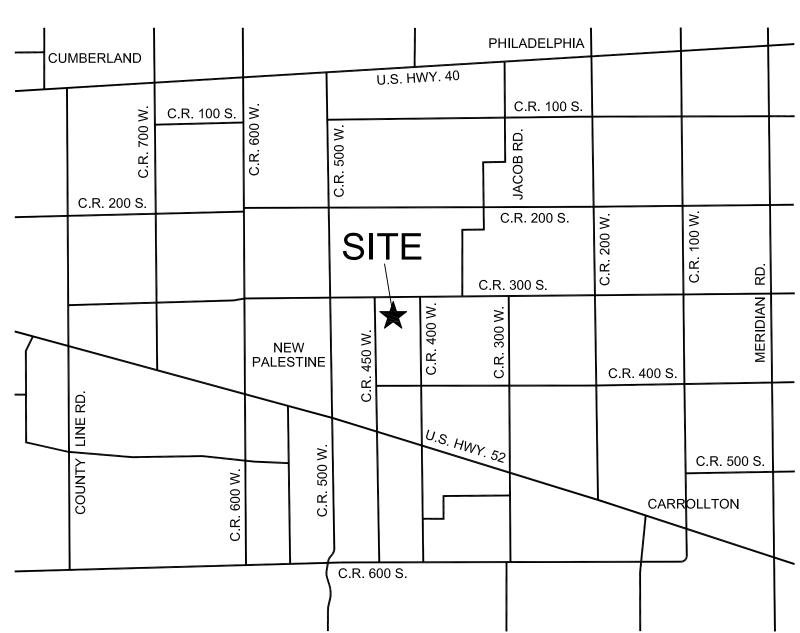
NO.	REVISION:	DATE:	AUTH.

CONSULTANT
COOR CONSULTING & LAND
SERVICES, CORPORATION
303 WEST MAIN STREET
KNIGHTSTOWN, IN 46148
(765) 345-5943

NOTES: THESE DOCUMENTS, PLANS, AND SPECIFICATIONS ARE SUBJECT TO PERIODIC UPDATES AND REVISIONS. THE CONTRACTOR, OWNER, AND SUPPLIERS ARE RESPONSIBLE FOR VERIFICATION OF THE MOST CURRENT ISSUE AND REVISION, PRIOR TO ANY USE.

(317) 462-7797





GENERAL NOTES

Covenant Note: The within project is subject to the declaration of covenants, conditions and restrictions for "The Overlook" as recorded in Instrument Number 030020064 in the Office of the Recorder of Hancock County, Indiana.

The within project is based in part upon the following: The Original Primary Plat dated December 26, 2001; the Amended Primary Plat dated April 23, 2002; The Overlook Section One as recorded in Plat Cabinet "C", Slide 133, Instrument Number 030020065; The Overlook Section One Certificate of Correction for lots 16-23 as recorded in Plat Cabinet "C", Slide 152, Instrument Number 040001558; The Overlook Section Two as recorded in Plat Cabinet "C", Slide 170, Instrument Number 040012915; and a Boundary Survey for "Hancock Land Company LLC." prepared by Gibson Surveying Group, dated May 22, 2002 and recorded in Instrument Number 0208316.

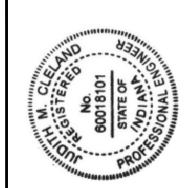
Basis of Bearings: The basis of bearings for this survey is an assumed bearing of North 00 degrees 03 minutes 40 seconds West along the West line of the Northeast Quarter of Section 20 per the aforementioned plats of "The Overlook".

Theory of Location: The lines and corners as established this survey are based upon monuments found marking the Quarter Section corners of the subject Section and monuments found at or near the corners of various lots within the subject plats and monuments found at or near the various corners of said prior survey and deeds of record.

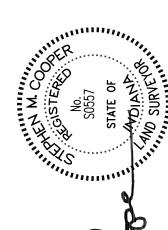
SECTION THREE LAND DESCRIPTION

(A part of Instrument Number 030002243)

A part of the West Half of the Northeast Quarter of Section 20, Township 15 North, Range 6 East, in Sugar Creek Township, Hancock County, Indiana, being more particularly described as follows: BEGINNING at the Northwest corner of said Northeast Quarter Section; thence North 89 degrees 53 minutes 48 seconds East along the North line thereof a distance of 488.03 feet; thence South 00 degrees 06 minutes 12 seconds East a distance of 66.50 feet; thence South 55 degrees 52 minutes 37 seconds West a distance of 211.56 feet; thence South 42 degrees 10 minutes 11 seconds West a distance of 142.29 feet; thence South 15 degrees 48 minutes 36 seconds West a distance of 110.41 feet; thence South 74 degrees 11 minutes 24 seconds East a distance of 340.00 feet; thence North 15 degrees 48 minutes 36 seconds East a distance of 27.16 feet; thence North 89 degrees 53 minutes 48 seconds East a distance of 307.91 feet to the West line of a tract of land described in Deed Record 137, Page 204 in the Office of the Recorder of Hancock County, Indiana; thence South 01 degrees 11 minutes 24 seconds East along the West line thereof a distance of 278.48 feet to the Southwest corner of said tract; thence North 67 degrees 59 minutes 27 seconds East along the South line thereof a distance of 46.18 feet to the West line of a tract of land described in Instrument Number 201702178 in said Recorder's Office; thence South 15 degrees 48 minutes 36 seconds West along the West line thereof a distance of 530.33 feet; thence South 05 degrees 41 minutes 24 seconds East along the West line thereof a distance of 83.76 feet to the Northeast corner of The Overlook Section One, as recorded in Plat Cabinet "C", Slide 133, Instrument Number 030020065 and the Certificate of Correction as recorded in Plat Cabinet "C", Slide 152, Instrument Number 040001558 in said Recorder's Office; thence South 88 degrees 19 minutes 07 seconds West along the North line thereof a distance of 15.04 feet to the Southeast corner of a tract of land described in Instrument Number 110007759 in said Recorder's Office; thence North 05 degrees 41 minutes 24 seconds West along the East line thereof a distance of 30.07 feet to the Northeast corner of said tract; thence South 88 degrees 19 minutes 07 seconds West along the North line thereof a distance of 192.39 feet to the Northwest corner of said tract, said point being on a curve having a radius of 263.00 feet, the radius point of which bears South 86 degrees 17 minutes 50 seconds East; thence Southerly along said curve to the left an arc distance of 30.04 feet to a point which bears South 87 degrees 09 minutes 36 seconds West from said radius point, said point being on the North line of said plat of The Overlook Section One; thence South 45 degrees 47 minutes 15 seconds West along the North line thereof a distance of 62.93 feet; thence North 74 degrees 50 minutes 24 seconds West continuing along the North line thereof a distance of 193.41 feet; thence North 01 degrees 03 minutes 40 seconds West a distance of 65.35 feet; thence North 15 degrees 48 minutes 36 seconds East a distance of 192.15 feet; thence North 74 degrees 11 minutes 24 seconds West a distance of 20.00 feet; thence North 15 degrees 48 minutes 36 seconds East a distance of 570.00 feet; thence North 74 degrees 11 minutes 24 seconds West a distance of 320.00 feet; thence South 15 degrees 48 minutes 36 seconds West a distance of 30.87 feet; thence South 88 degrees 56 minutes 20 seconds West a distance of 155.60 feet to the West line of said Northeast Quarter; thence North 01 degrees 03 minutes 40 seconds West along the West line thereof a distance of 480.00 feet to the POINT OF BEGINNING. Containing 11.715 acres, more or



udith M. Cleland leland Environmental Engineering, Inc. 308 Thorn Bend Drive, Indianapolis, IN 462.



Stephen M. Cooper, P.S. # \$0557
Dated: 9/24/18

FILE #: 2017-124.BASE.DWG

DATE: 9/24/18

APPROVED BY: SMC

Steph

DRAWN BY: MTL

Dated

HANCOCK LAND CO., LLC

PART OF W. 1/2 - N.E. 1/4

S 20 - T 15 N - R 06 E

SUGAR CREEK TOWNSHIP

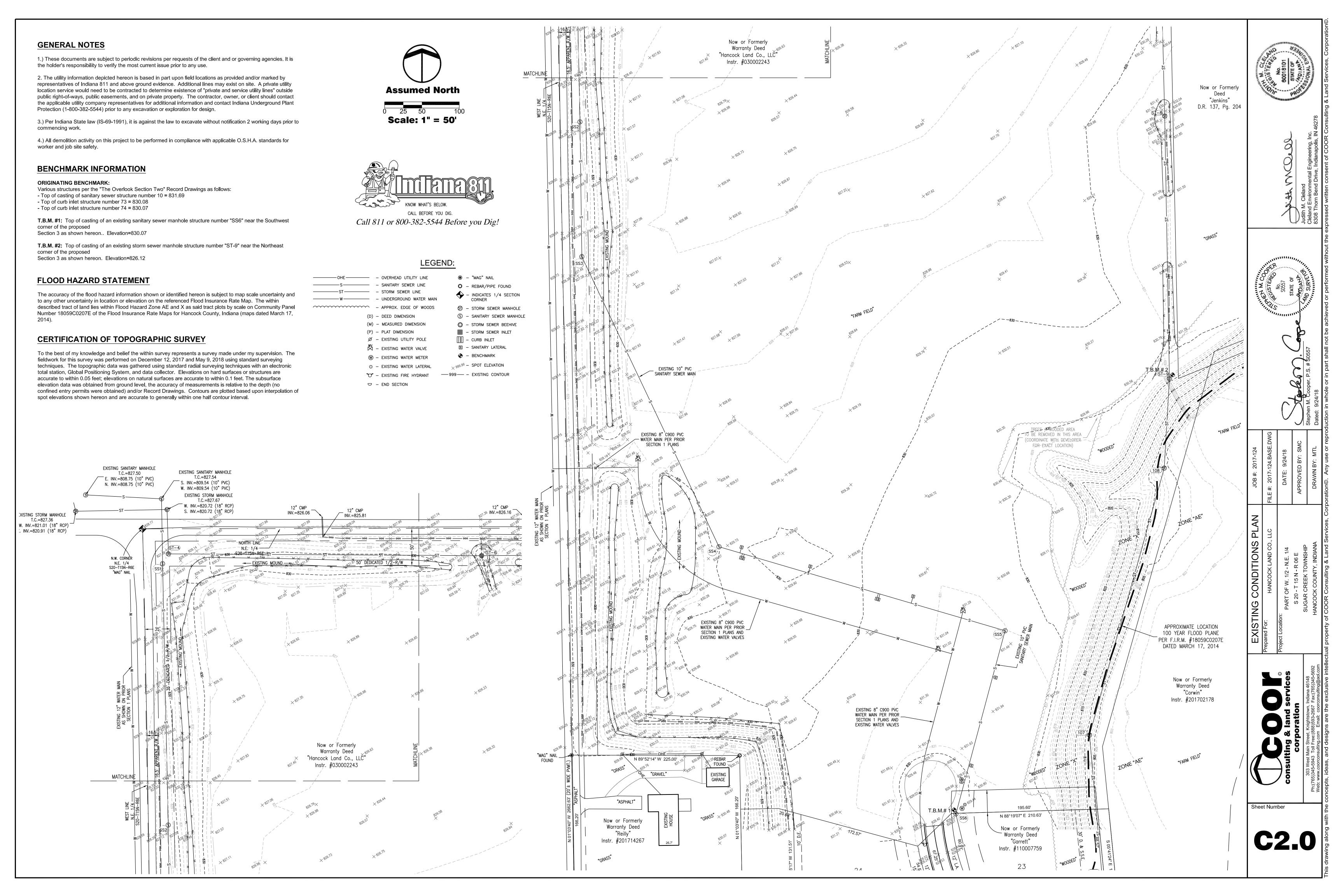
ulting & land services corporation

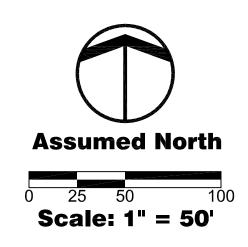
Vest Main Street, Knightstown, Indiana 46148

Toll Free: (8881503-2562 Fax:7765)345-5602

Sheet Number

C1.0





SIDE AND REAR YARD RESTRICTIONS REAR YARD SETBACK

REAR YARD SETBACK
SIDE YARD SETBACK
MIN. DISTANCE BETWEEN STRUCTURES
15 MIN.
7' MIN. WITH 19' TOTAL AGG.
19'

STREET INFORMATION ALL STREETS WITHIN BLOCK "K"ARE PRIVATE. THE OVERLOOK HOMEOWNERS

ALL STREETS WITHIN BLOCK "K"ARE PRIVATE. THE OVERLOOK HOMEOWNERS ASSOCIATION WILL MAINTAIN STREETS. APPARENT ACREAGE WITHIN BLOCK "K" IS 1.224 ACRES (54,138 SQ. FT.). THE AFOREMENTIONED BLOCKS CONTAIN 12' WIDE LANDSCAPE AREAS (12' L.A.) AS SHOWN HEREON.

THE WITHIN PLAT IS SUBJECT TO THE DECLARATION OF COVENANTS, CONDITIONS AND RESTRICTIONS FOR "THE OVERLOOK" AS RECORDED IN INSTRUMENT NUMBER 030020064 IN THE OFFICE OF THE RECORDER OF HANCOCK COUNTY, INDIANA.



Call 811 or 800-382-5544 Before you Dig!

LEGEND:

 OHE
 — OVERHEAD UTILITY LINE

 S
 — SANITARY SEWER LINE

 ST
 — STORM SEWER LINE

— UNDERGROUND WATER MAIN
 — APPROX. FDGF OF WOODS

- APPROX. EDGE OF WOODS

(D) - DEED DIMENSION

(D) - DEED DIMENSION

(M) - MEASURED DIMENSION

(C) - DEED DIMENSION

(P) - PLAT DIMENSION

Ø - EXISTING UTILITY POLE

WY - EXISTING WATER VALVE

W − EXISTING WATER VALVE
 W − EXISTING WATER METER
 ⊙ − EXISTING WATER LATERAL

END SECTION

- INDICATES 1/4 SECTION CORNER

STORM SEWER MANHOLE

O - REBAR/PIPE FOUND

S - SANITARY SEWER MANHOLE⊕ - STORM SEWER BEEHIVEⅢ - STORM SEWER INLET

□ − CURB INLET
□ − SANITARY LATERAL

⊕ − BENCHMARK

 \odot - EXISTING WATER LATERAL $\times 9^{99}$ - SPOT ELEVATION \longrightarrow - EXISTING FIRE HYDRANT - 999 - EXISTING CONTOUR

PROPOSED LEGEND:

PROPOSED STORM STRUCTURE NO.
 PROPOSED STORM INLET
 PROPOSED STORM END SECTION

999.9 - PROPOSED SPOT ELEVATION
9.99 - PROPOSED PAVEMENT ELEVATION

STORM SEWER OVERFLOW PATHA - A.D.A. HANDICAP RAMP

(B) - 2' ROLL CURB

S.S.D.&U.E.

1' 4' 12' L.A.

13' 26'

CONC.
WALK

VARIES

SANITARY SEWER MAIN
(LOCATION TYPICAL)

PIPE UNDERDRAIN
(TYPICAL)

PIPE UNDERDRAIN
(TYPICAL)

PIPE UNDERDRAIN
(TYPICAL)

PIPE UNDERDRAIN
(TYPICAL)

OF CURPE

TYPICAL STREET SECTION (NO SCALE)

GENERAL PLAN NOTES

1.) These documents are subject to periodic revisions per requests of the client and or governing agencies. It is the holder's responsibility to verify the most current issue prior to any use.

2.) Contractor shall recognize respective work and responsibility to verify location, size, and elevation of existing utilities, structures, pipes, pavements, etc. as related to their work. Notify architect/engineer of any conflict and/or discrepancies in the construction documents prior to the start of construction.

3.) The contractor shall be responsible for obtaining or verifying that all permits and approvals are obtained from the respective city, county, and state agencies prior to any construction activities.

4.) It is the contractor's responsibility to acquaint himself with subsoil conditions. Contractor is responsible for all cut/fill materials required to match compacted design subgrade. Topsoil removal limits to be determined by the contractor and/or owner.

5.) The plans show the location of all known utilities located within the limits of the contract according to information provided by the various utility companies, previous construction plans and as evidenced by observation of above ground conditions by the surveyor. The accuracy of this information is not guaranteed.

6.) All pavement patching due to utilities installation, construction of curbs, etc., or damage to existing pavement during construction shall be patched with a pavement section which meets or exceeds the specifications by the city or specifications by INDOT.

7.) All existing manholes, catch basin grates, water valves, gas valves, etc., shall be adjusted to proposed finish grade elevation as necessary.

8.) All pipe lengths shown on drawings are for hydraulic calculation purposes only. It is the responsibility of the contractor to determine exact lengths required for actual installation.

9.) Construction of all sewer lines and structures shall be in accordance with local and state code rules and regulations.

10.) Repair, replace or maintain any/all existing utilities within limits of construction.

size shall be per the direction of the governing agency.

a required minimum.

11.) Handicapped ramps and signage shall be in accordance with federal, state, county, city and local codes whichever has jurisdiction. See site details for specifications.

12.) The contractor shall be responsible to provide at his expense all automobile and pedestrian traffic control devices required by federal, state, city or local agencies. The mount, location and

13.) It is the responsibility of the contractor to remove all mud, dirt, gravel and other materials tracked onto any public or private streets or sidewalks. The contractor must clean these daily if necessary. The contractor must use water or other expectable methods to keep airborne dust to

14.) Coordinate all work with county, city or governing utility at least 48 hours prior to the start of

15.) All construction on this site to be performed in compliance with O.S.H.A. standards for worker safety.

16.) The proposed elevations depicted hereon are based upon existing surface conditions as of the date of the within survey. No information about fluctuating water tables, soil conditions, or soil types within the subject tract has been provided by client or others unless otherwise noted. If any groundwater is witnessed during the construction process, Coor Consulting & Land Services, Corp. should be notified immediately and additional construction techniques may be incorporated to allocate future problems.

17.) Provide positive drainage without ponding in all paved areas after installation of asphalt base. Contractor to test for and correct any ponding conditions prior to final coat.

18.) Match existing grade at boundary line.

19.) Fill material shall consist of earth obtained from cut areas, borrow pits or other approved sources. Earth shall be free from organic matter and other deleterious substances and large rocks. The fill material shall be placed in layers not to exceed six inches following compaction, proper moisture content of fill material will be such to achieve specified compaction density. All fill beneath paved areas, floor slabs and future buildings shall be compacted to at least 95% of the maximum dry density per ASTM D-1557. Field compacting test shall be run on each lift, in fill sections, and the required compaction on each lift shall be in accordance with INDOT Section

20.) The contractor shall coordinate all earthwork quantities prior to the start of construction. If an excess or shortage of earth is encountered, the contractor shall coordinate with the owner and designer the requirements for stockpiling, removal or importing of earth.

21.) Verify existing inverts of storm sewer and adjust as required. Document any changes.

22.) Follow all local and state codes in reference to domestic line installation and sanitary lateral installation.

23.) The contractor shall field verify the location and the pipe invert depth where the proposed connection is made to the existing sanitary sewer.

24.) All water mains to be 54" minimum below grade.

meter with gas company.

25.) The contractor shall contact all applicable utilities and verify any and all fees associated with the installation of all utilities.

26.) Contractor to provide pipe bedding and compact to 95% density of standard proctor in paved areas. Pipe bedding in non-paved areas shall be compacted to 85% density of standard proctor.

27.) Coordinate location of electrical and telephone lines with local utilities and electrical drawings.

28.) Coordinate location and size of gas service connection and installation of service line and

29.) Maintain a minimum of 10' horizontal clearance between sanitary lateral line and domestic line service.

30.) All construction specifications should comply with current specifications.

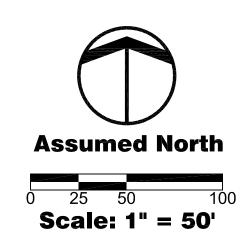
31.) Trenches under paved areas (excluding sidewalks) shall be backfilled with granular material "MAG" NAIL and compacted in lifts. Granular material shall extend 5 feet beyond the limits of the paved area.

32.) See Plat for lot dimensions, building lines, and easements.

33.) Deep water signs to be placed as shown per development plan.

34.) All landscaping, buffering, and/or screening for the proposed development to match existing Section One, Section Two and County Specifications.





SIDE AND REAR YARD RESTRICTIONS REAR YARD SETBACK

7' MIN. WITH 19' TOTAL AGG. SIDE YARD SETBACK MIN. DISTANCE BETWEEN STRUCTURES

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LEGEND:

- OVERHEAD UTILITY LINE SANITARY SEWER LINE — – STORM SEWER LINE

— – UNDERGROUND WATER MAIN - APPROX. EDGE OF WOODS

END SECTION

(D) - DEED DIMENSION

(M) - MEASURED DIMENSION (P) - PLAT DIMENSION \emptyset - EXISTING UTILITY POLE

WV - EXISTING WATER VALVE W - EXISTING WATER METER

- CURB INLET SANITARY LATERAL BENCHMARK ⊙ – EXISTING WATER LATERAL

ygg.99 - SPOT ELEVATION <equation-block>

O - REBAR/PIPE FOUND

- INDICATES 1/4 SECTION

STORM SEWER MANHOLE

— STORM SEWER BEEHIVE

■ - STORM SEWER INLET

S - SANITARY SEWER MANHOLE

XISTING STORM MANHOLE

T.C.=827.36 v. INV.=821.01 (18" RCP)

INV.=820.91 (18" RCP)

PROPOSED LEGEND:

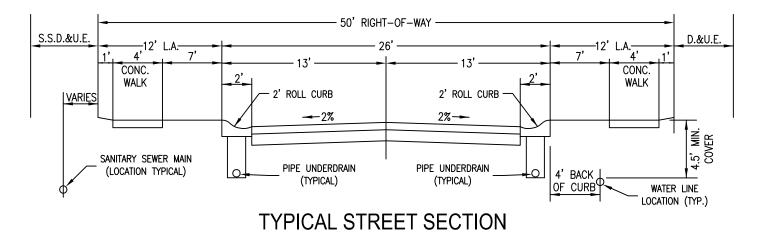
——999— – PROPOSED CONTOUR —● (SS#) - PROPOSED SANITARY MANHOLE

(###) - PROPOSED STORM STRUCTURE NO. (- PROPOSED STORM END SECTION

- PROPOSED SPOT ELEVATION - PROPOSED PAVEMENT ELEVATION - STORM SEWER OVERFLOW PATH

A – A.D.A. HANDICAP RAMP

(B) - 2' ROLL CURB



(NO SCALE)

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22.) Follow all local and state codes in reference to domestic line installation and sanitary lateral installation.

23.) The contractor shall field verify the location and the pipe invert depth where the proposed connection is made to the existing sanitary sewer.

24.) All water mains to be 54" minimum below grade.

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27.) Coordinate location of electrical and telephone lines with local utilities and electrical drawings.

28.) Coordinate location and size of gas service connection and installation of service line and meter with gas company.

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30.) All construction specifications should comply with current specifications.

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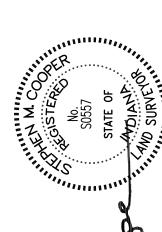
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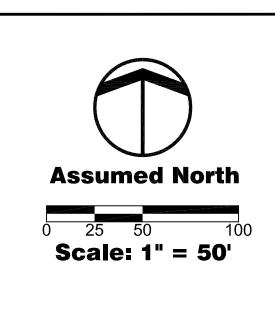
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EXISTING STORM MANHOLE T.C.=827.67 W. INV.=820.72 (18" RCP) 12" CMP INV.=826.06 S. INV.=820.72 (18" RCP) __ INV.=825.81 N.W. CORNÈR N.E. 1/4 S20-T15N-R6E "MAG" NAIL BLOCK "N" C.A., D.&U.E., L.E. POND N.P. ELEV=821.00 10 YR. ELEV=821.80 100 YR. ELEV=823.1 BOTTOM ELEV=811.00 TOP OF BANK=826,00 Warranty Deed Hancock Land Co., LLĈ Instr. #030002243 MATCHLINE __±26 L.F. OF 24" RCP AT 0.38% INV.=826.9 STREET R/W 829.8 PAD=831.0 PAD=831.0 829.0 FL/HP PAD=831.0 __/8" BEEHIVÉ PAD=830.5 EXISTING 8" C900 PVC - WATER MAIN PER PRIOR 828.7 FL~ 12" RCP AT 0.38% (ISTING 12" WATER MAI AS SHOWN ON PRIOR SECTION 1 PLANS -820 - 819 V 8" BEEHIVE PAD=830.5 INV.=826.6







LEGEND: OHE - OVERHEAD UTILITY LINE - SANITARY SEWER LINE - STORM SEWER LINE

- APPROX. EDGE OF WOODS

(D) - DEED DIMENSION

U - EXISTING FIRE HYDRANT

END SECTION

(M) - MEASURED DIMENSION (P) - PLAT DIMENSION

 \varnothing - EXISTING UTILITY POLE WV - EXISTING WATER VALVE

- STORM SEWER INLET CURB INLET (W) - EXISTING WATER METER $_{\searrow 99}$.99 – SPOT ELEVATION ⊙ − EXISTING WATER LATERAL

S – SANITARY LATERAL BENCHMARK

PROPOSED LEGEND:

— (SS#) - PROPOSED SANITARY MANHOLE

- PROPOSED STORM STRUCTURE NO.

- PROPOSED STORM END SECTION

- PROPOSED PAVEMENT ELEVATION

CALL BEFORE YOU DIG. Call 811 or 800-382-5544 Before you Dig!

- PROPOSED SPOT ELEVATION

- PROPOSED STORM INLET

- STORM SEWER OVERFLOW PATH

(A) – A.D.A. HANDICAP RAMP

(B) - 2' ROLL CURB

——999— – PROPOSED CONTOUR

O - REBAR/PIPE FOUND

- INDICATES 1/4 SECTION

⑤ - STORM SEWER MANHOLE

STORM SEWER BEEHIVE

S - SANITARY SEWER MANHOLE

EXISTING STORM SEWER STRUCTURE TABLE:

EXISTING STORM MANHOLE EXISTING STORM INLET EXISTING STORM MANHOLE EXISTING STORM MAN	
T.C.=827.80 T.C.=823.95 T.C.=827.21 T.C.=831.75	IHOLE
N. INV.=820.20 (18" RCP) W. INV.=819.45 (18" RCP) W. INV.=819.01 (18" RCP) N. INV.=818.65 (18" E. INV.=820.10 (18" RCP) E. INV.=819.45 (18" RCP) S. INV.=818.91 (18" RCP) S. INV.=818.55 (18"	
2. INV020.10 (10 Not) 2. INV010.33 (10 Not) 3. INV010.33 (10 Not)	NOI)

106 ST-9 STORM MANHOLE PER EXISTING STORM MANHOLE STORM MANHOLE PER STORM MANHOLE PER T.C.=826.12 PRIOR PROPOSED PLANS PRIOR PROPOSED PLANS PRIOR PROPOSED PLANS T.C.=806.50 N. INV.=817.87 (18" RCP) N. INV.=799.17 (30" N-12) N. INV.=801.04 (18" RCP) N. INV.=799.83 (30" N-12) S. INV.=809.87 (18" RCP) W. INV.=801.04 (18" RCP) S. INV.=799.73 (30" N-12) S. INV.=799.07 (30" N-12) W. INV.=800.67 S. INV.=800.94 (30" N-12)

EXISTING SANITARY SEWER STRUCTURE TABLE:

EXISTING SANITARY MANHOLE EXISTING SANITARY MANHOLE EXISTING SANITARY MANHOLE EXISTING SANITARY MANHOLE T.C.=828.25 T.C.=827.84 T.C.=828.08 T.C.=829.65 S. INV.=811.35 (10" PVC) S. INV.=812.54 (10" PVC) S.E. INV.=813.28 (10" PVC) S.E. INV.=814.50 (10" PVC) N. INV.=811.35 (10" PVC) N. INV.=812.49 (10" PVC) N. INV.=812.98 (10" PVC) N.W. INV.=814.50 (10" PVC) APPARENT E. INV.=815.03

EXISTING SANITARY MANHOLE EXISTING SANITARY MANHOLE T.C.=831.06 S. INV.=816.87 (10" PVC) N. INV.=822.46 (8" PVC) N. DROP INV.=816.36 N.E. INV.=816.67 (10" PVC) N.W. INV.=815.86 (10" PVC) S.W. INV.=815.96 (10" PVC)

GENERAL PLAN NOTES

code rules and regulations.

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3.) The contractor shall be responsible for obtaining or verifying that all permits and approvals are obtained from the respective city, county, and state agencies prior to any construction activities.

4.) It is the contractor's responsibility to acquaint himself with subsoil conditions. Contractor is responsible for all cut/fill materials required to match compacted design subgrade. Topsoil removal limits to be determined by the contractor and/or owner.

5.) The plans show the location of all known utilities located within the limits of the contract according to information provided by the various utility companies, previous construction plans and as evidenced by observation of above ground conditions by the surveyor. The accuracy of this information is not guaranteed.

6.) All pavement patching due to utilities installation, construction of curbs, etc., or damage to existing pavement during construction shall be patched with a pavement section which meets or exceeds the specifications by the city or specifications by INDOT.

7.) All existing manholes, catch basin grates, water valves, gas valves, etc., shall be adjusted to proposed finish grade elevation as necessary.

8.) All pipe lengths shown on drawings are for hydraulic calculation purposes only. It is the responsibility of the contractor to determine exact lengths required for actual installation.

9.) Construction of all sewer lines and structures shall be in accordance with local and state

10.) Repair, replace or maintain any/all existing utilities within limits of construction.

11.) Handicapped ramps and signage shall be in accordance with federal, state, county, city and local codes whichever has jurisdiction. See site details for specifications.

12.) The contractor shall be responsible to provide at his expense all automobile and pedestrian traffic control devices required by federal, state, city or local agencies. The mount, location and size shall be per the direction of the governing agency.

13.) It is the responsibility of the contractor to remove all mud, dirt, gravel and other materials tracked onto any public or private streets or sidewalks. The contractor must clean these daily if necessary. The contractor must use water or other expectable methods to keep airborne dust to a required minimum.

14.) Coordinate all work with county, city or governing utility at least 48 hours prior to the start of

15.) All construction on this site to be performed in compliance with O.S.H.A. standards for

16.) The proposed elevations depicted hereon are based upon existing surface conditions as of the date of the within survey. No information about fluctuating water tables, soil conditions, or soil types within the subject tract has been provided by client or others unless otherwise noted. If any groundwater is witnessed during the construction process, Coor Consulting & Land Services, Corp. should be notified immediately and additional construction techniques may be incorporated to allocate future problems.

17.) Provide positive drainage without ponding in all paved areas after installation of asphalt base. Contractor to test for and correct any ponding conditions prior to final coat.

18.) Match existing grade at boundary line.

19.) Fill material shall consist of earth obtained from cut areas, borrow pits or other approved sources. Earth shall be free from organic matter and other deleterious substances and large rocks. The fill material shall be placed in layers not to exceed six inches following compaction, proper moisture content of fill material will be such to achieve specified compaction density. All fill beneath paved areas, floor slabs and future buildings shall be compacted to at least 95% of the maximum dry density per ASTM D-1557. Field compacting test shall be run on each lift, in fill sections, and the required compaction on each lift shall be in accordance with INDOT Section

20.) The contractor shall coordinate all earthwork quantities prior to the start of construction. If an excess or shortage of earth is encountered, the contractor shall coordinate with the owner and designer the requirements for stockpiling, removal or importing of earth.

21.) Verify existing inverts of storm sewer and adjust as required. Document any changes.

22.) Follow all local and state codes in reference to domestic line installation and sanitary lateral installation.

23.) The contractor shall field verify the location and the pipe invert depth where the proposed connection is made to the existing sanitary sewer.

24.) All water mains to be 54" minimum below grade.

25.) The contractor shall contact all applicable utilities and verify any and all fees associated with the installation of all utilities.

26.) Contractor to provide pipe bedding and compact to 95% density of standard proctor in paved areas. Pipe bedding in non-paved areas shall be compacted to 85% density of standard proctor.

27.) Coordinate location of electrical and telephone lines with local utilities and electrical drawings.

28.) Coordinate location and size of gas service connection and installation of service line and meter with gas company.

29.) Maintain a minimum of 10' horizontal clearance between sanitary lateral line and domestic line service.

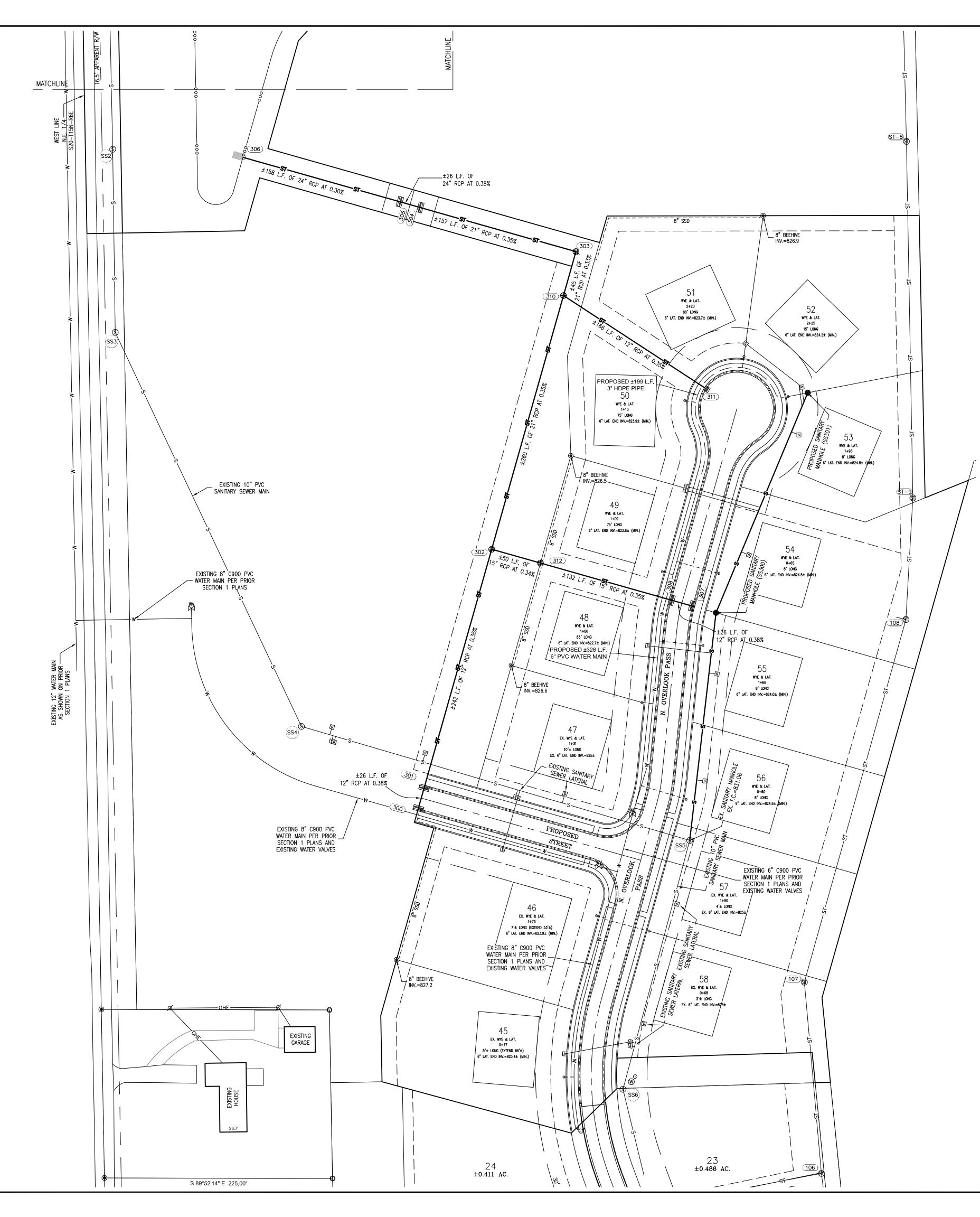
30.) All construction specifications should comply with current specifications.

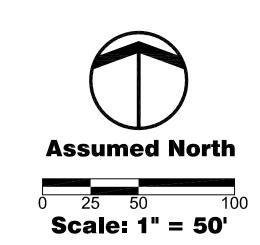
31.) Trenches under paved areas (excluding sidewalks) shall be backfilled with granular material and compacted in lifts. Granular material shall extend 5 feet beyond the limits of the paved area.

32.) See Plat for lot dimensions, building lines, and easements.

33.) Deep water signs to be placed as shown per development plan.

34.) All landscaping, buffering, and/or screening for the proposed development to match existing Section One, Section Two and County Specifications.





OHE - OVERHEAD UTILITY LINE - SANITARY SEWER LINE — – STORM SEWER LINE

- APPROX. EDGE OF WOODS (D) - DEED DIMENSION

END SECTION

ST-8

S. INV.=799.07 (30" N-12)

W. INV.=800.67

(M) - MEASURED DIMENSION (P) - PLAT DIMENSION

LEGEND:

 \varnothing - EXISTING UTILITY POLE WY - EXISTING WATER VALVE (W) - EXISTING WATER METER ⊙ − EXISTING WATER LATERAL U - EXISTING FIRE HYDRANT

O - REBAR/PIPE FOUND

- INDICATES 1/4 SECTION

⑤ - STORM SEWER MANHOLE S - SANITARY SEWER MANHOLE STORM SEWER BEEHIVE

- STORM SEWER INLET UB - CURB INLET S – SANITARY LATERAL

BENCHMARK $_{\searrow 99}$.99 – SPOT ELEVATION

EXISTING STORM SEWER STRUCTURE TABLE:

(ST-7)

S. INV.=799.73 (30" N-12)

EXISTING STORM MANHOLE	EXISTING STORM INLET	EXISTING STORM MANHOLE	EXISTING STORM MANHOLE
T.C.=827.80	T.C.=823.95	T.C.=827.21	T.C.=831.75
N. INV.=820.20 (18" RCP)	W. INV.=819.45 (18" RCP)	W. INV.=819.01 (18" RCP)	N. INV.=818.65 (18" RCP)
E. INV.=820.10 (18" RCP)	E. INV.=819.45 (18" RCP)	S. INV.=818.91 (18" RCP)	S. INV.=818.55 (18" RCP)
(ST-9)	(108)	(107)	106
EXISTING STORM MANHOLE	STORM MANHOLE PER	STORM MANHOLE PER	STORM MANHOLE PER
T.C.=826.12	PRIOR PROPOSED PLANS	PRIOR PROPOSED PLANS	PRIOR PROPOSED PLANS
N. INV.=817.87 (18" RCP)	T.C.=806.50	T.C.=806.50	T.C.=804.50
S. INV.=809.87 (18" RCP)	N. INV.=801.04 (18" RCP)	N. INV.=799.83 (30" N-12)	N. INV.=799.17 (30" N-12)

W. INV.=801.04 (18" RCP)

S. INV.=800.94 (30" N-12)

EXISTING SANITARY SEWER STRUCTURE TABLE:

(SS1)	(SS2)	(SS3)	(SS4)
EXISTING SANITARY MANHOLE T.C.=828.25	EXISTING SANITARY MANHOLE T.C.=827.84	EXISTING SANITARY MANHOLE T.C.=828.08	EXISTING SANITARY MANHOL T.C.=829.65
S. INV.=811.35 (10" PVC) N. INV.=811.35 (10" PVC)	S. INV.=812.54 (10" PVC) N. INV.=812.49 (10" PVC)	S.E. INV.=813.28 (10" PVC) N. INV.=812.98 (10" PVC) APPARENT E. INV.=815.03	S.E. INV.=814.50 (10" PVC N.W. INV.=814.50 (10" PVC

EXISTING SANITARY MANHOLE EXISTING SANITARY MANHOLE T.C.=831.06 N. INV.=822.46 (8" PVC) S. INV.=816.87 (10" PVC) N. DROP INV.=816.36 N.E. INV.=816.67 (10" PVC) N.W. INV.=815.86 (10" PVC) S.W. INV.=815.96 (10" PVC)

GENERAL PLAN NOTES

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ST-4

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15.) All construction on this site to be performed in compliance with O.S.H.A. standards for

PROPOSED LEGEND:

——999— – PROPOSED CONTOUR — (SS#) - PROPOSED SANITARY MANHOLE - PROPOSED STORM STRUCTURE NO. - PROPOSED STORM INLET - PROPOSED STORM END SECTION - PROPOSED SPOT ELEVATION

> - PROPOSED PAVEMENT ELEVATION - STORM SEWER OVERFLOW PATH (A) – A.D.A. HANDICAP RAMP (B) - 2' ROLL CURB

CALL BEFORE YOU DIG. Call 811 or 800-382-5544 Before you Dig!

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28.) Coordinate location and size of gas service connection and installation of service line and meter with gas company.

29.) Maintain a minimum of 10' horizontal clearance between sanitary lateral line and domestic line service.

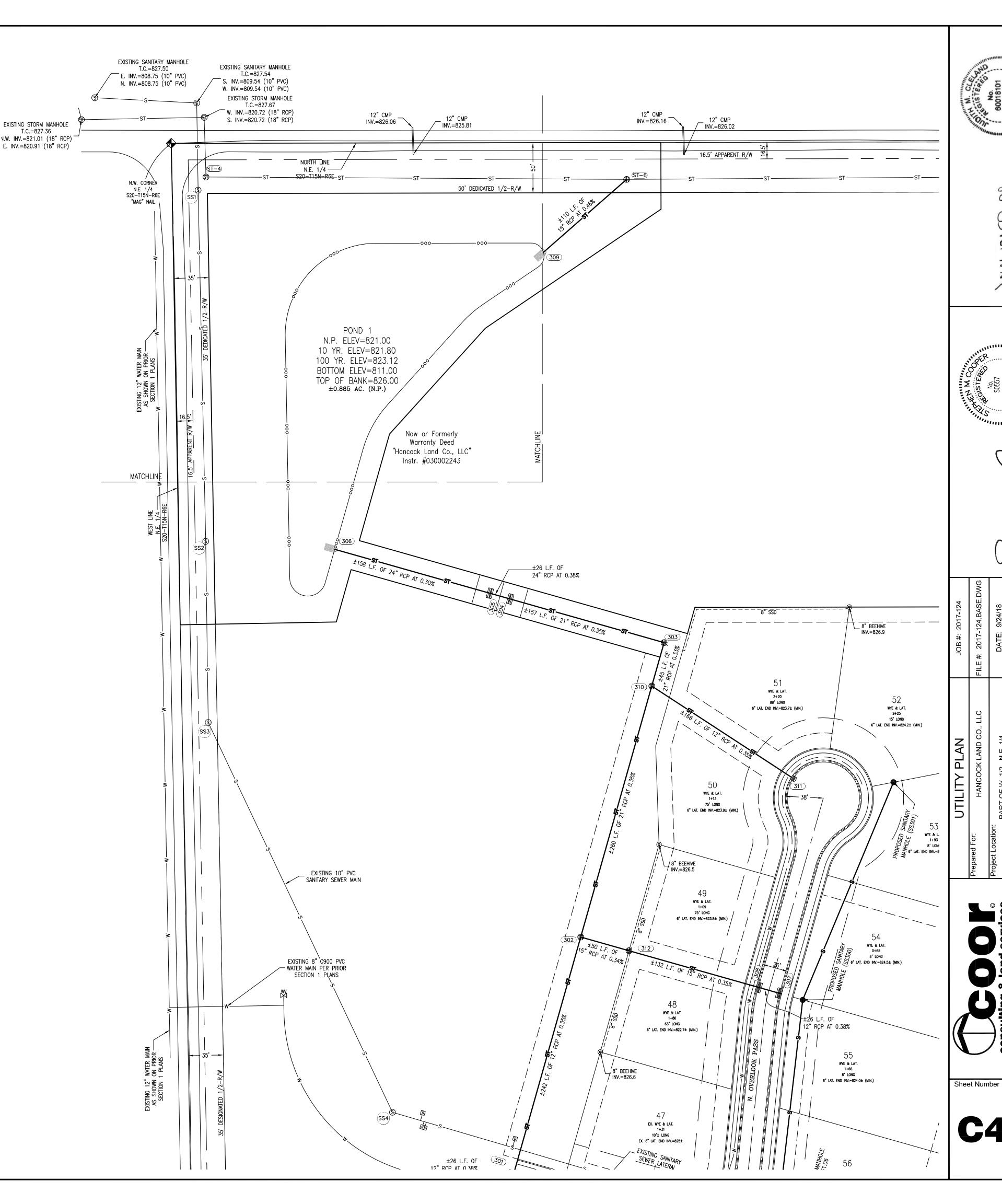
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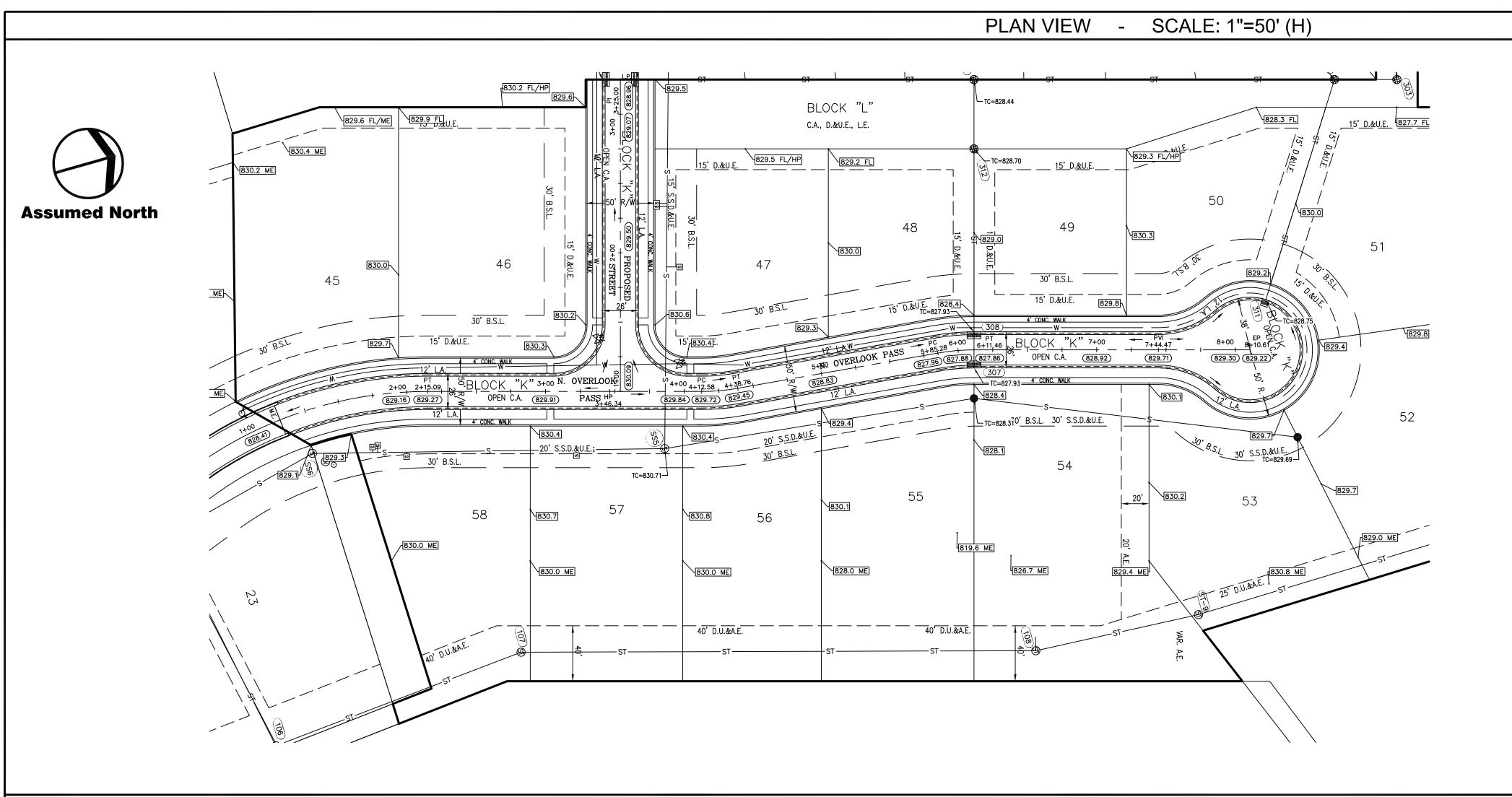
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OHE	 OVERHEAD UTILITY LINE
s	- SANITARY SEWER LINE
ST	- STORM SEWER LINE
w	- UNDERGROUND WATER MAIN
	- APPROX. EDGE OF WOODS

(D) - DEED DIMENSION (M) - MEASURED DIMENSION (P) - PLAT DIMENSION \emptyset - EXISTING UTILITY POLE

WY - EXISTING WATER VALVE W - EXISTING WATER METER ⊙ - EXISTING WATER LATERAL 💙 — EXISTING FIRE HYDRANT

END SECTION

O - REBAR/PIPE FOUND

- INDICATES 1/4 SECTION CORNER ⑤ - STORM SEWER MANHOLE S - SANITARY SEWER MANHOLE STORM SEWER BEEHIVE

■ - STORM SEWER INLET - CURB INLET S - SANITARY LATERAL 🕀 – BENCHMARK

, 999.99 - SPOT ELEVATION

PROPOSED LEGEND:

—● (SS#) - PROPOSED SANITARY MANHOLE - PROPOSED STORM STRUCTURE NO.

- PROPOSED STORM INLET

- PROPOSED STORM END SECTION 999.9 - PROPOSED SPOT ELEVATION

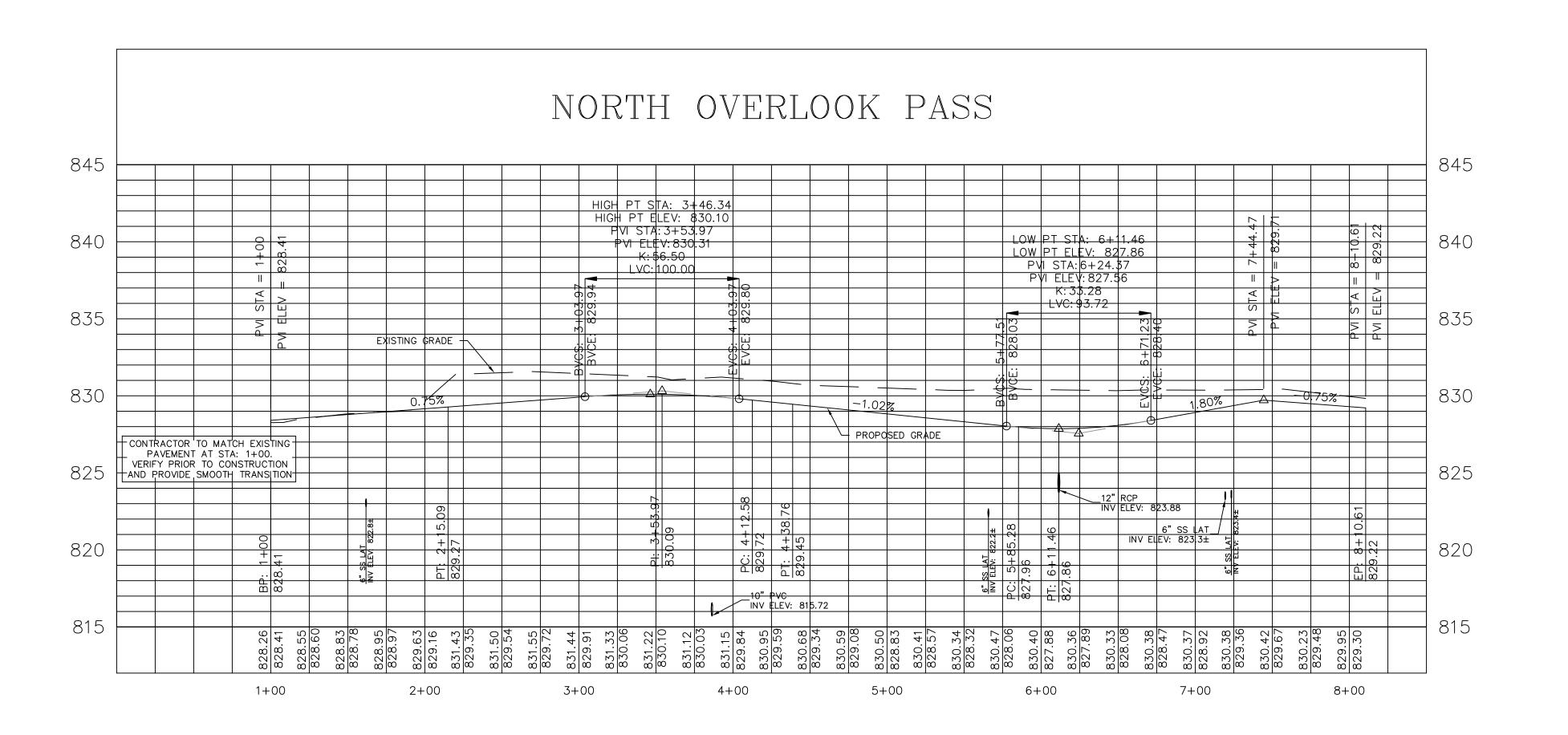
- PROPOSED PAVEMENT ELEVATION - STORM SEWER OVERFLOW PATH

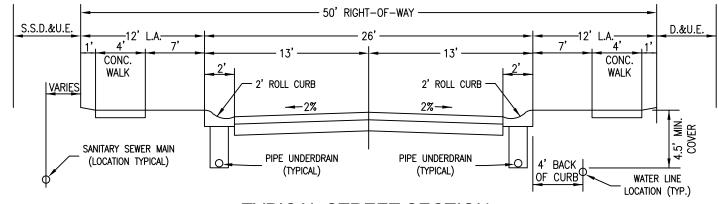
A – A.D.A. HANDICAP RAMP B - 2' ROLL CURB

CALL BEFORE YOU DIG.

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PROFILE VIEW SCALE: 1"=50' (H) 1"=5' (V)





TYPICAL STREET SECTION

1.) 165 pounds per square yard of HMA surface course on 330 pounds per square yard of HMA Intermediate course on 6" of

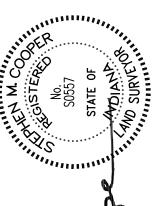
2.) Surface Aggregate Type A mixture, the surface aggregate requirements shall meet the requirements for less than 3,000,000

3.) Preparation of subgrade for Street Pavements shall be prepared per Section 215, Chemical Modification of Subgrade Soils of the Standard Specifications.

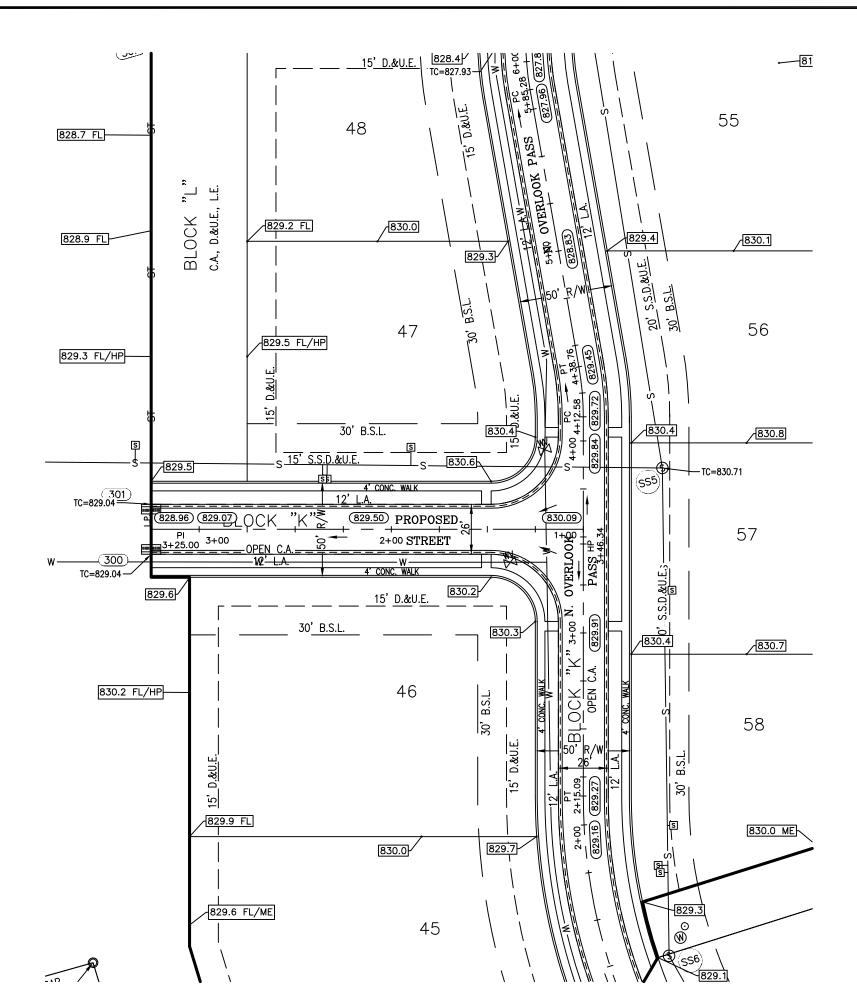
4.) All streets to be asphalt, see Development Plan and Street Plan & Profile sheets for designated street widths.

5.) Soil stabilization is required for all road subgrade.

6.) All sidewalks within the Hancock County right-of-way shall be four (4) feet wide concrete sidewalks of 4,000 psi concrete, four (4) inches thick sloped toward the street with expansion joints each forty-eight (48) inches along the entire street frontage of the proposed lots. The cross slope for all ramps is to be 1%, but a maximum of 2% is allowed by ADA standards.







SANITARY LATERAL

CURB INLET

OHE - OVERHEAD UTILITY LINE O - REBAR/PIPE FOUND - INDICATES 1/4 SECTION CORNER - APPROX. EDGE OF WOODS (S) - STORM SEWER MANHOLE S - SANITARY SEWER MANHOLE (D) - DEED DIMENSION STORM SEWER BEEHIVE

(M) - MEASURED DIMENSION (P) - PLAT DIMENSION \varnothing - EXISTING UTILITY POLE WY - EXISTING WATER VALVE

\varTheta – BENCHMARK W - EXISTING WATER METER ygg.99 - SPOT ELEVATION ⊙ − EXISTING WATER LATERAL 💙 – EXISTING FIRE HYDRANT U - END SECTION

PROPOSED LEGEND:

——999— - PROPOSED CONTOUR

- SS#) - PROPOSED SANITARY MANHOLE

- PROPOSED STORM STRUCTURE NO.

☐ − PROPOSED STORM END SECTION

999.9 - PROPOSED SPOT ELEVATION

- STORM SEWER OVERFLOW PATH

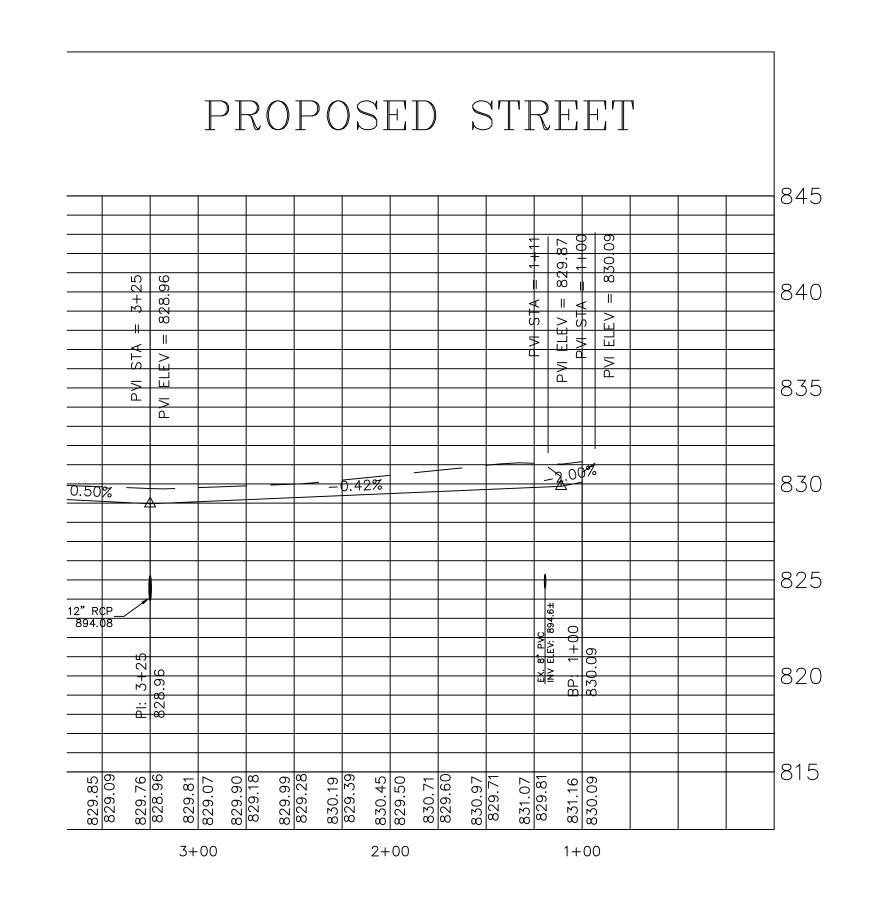
999.99 - PROPOSED PAVEMENT ELEVATION

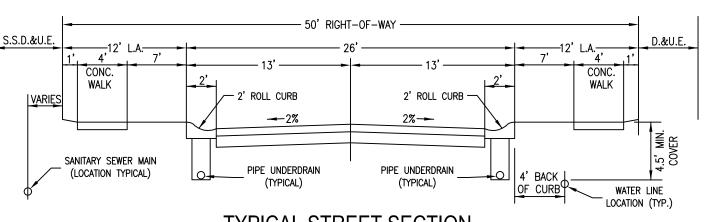
A – A.D.A. HANDICAP RAMP

B - 2' ROLL CURB



PROFILE VIEW - SCALE: 1"=50' (H) 1"=5' (V)





TYPICAL STREET SECTION (NO SCALE)

1.) 165 pounds per square yard of HMA surface course on 330 pounds per square yard of HMA Intermediate course on 6" of compacted #53 stone.

2.) Surface Aggregate Type A mixture, the surface aggregate requirements shall meet the requirements for less than 3,000,000 ESAL in 904.03(d).

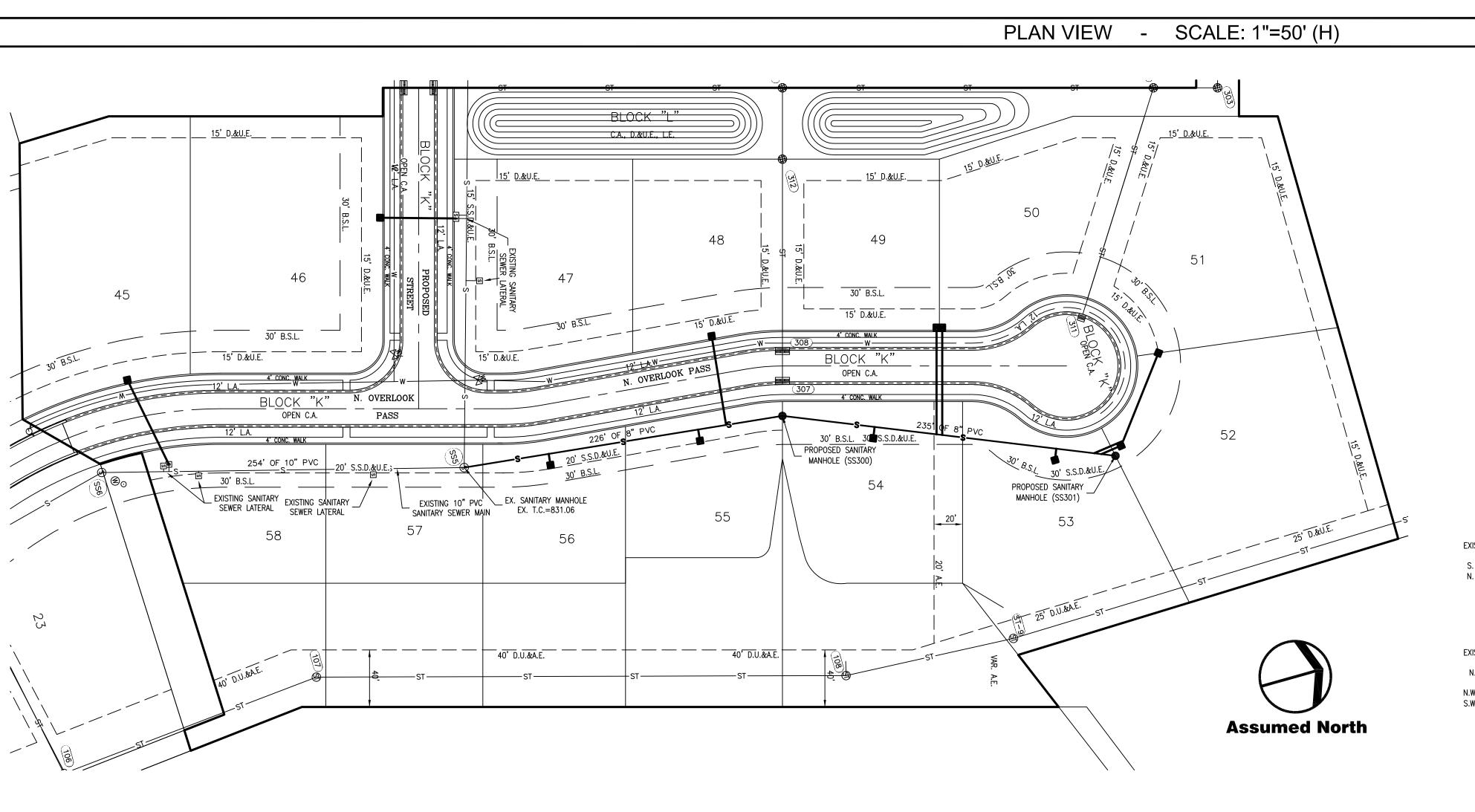
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O - REBAR/PIPE FOUND

- INDICATES 1/4 SECTION

-OHE - OVERHEAD UTILITY LINE - STORM SEWER LINE — – UNDERGROUND WATER MAIN - APPROX. EDGE OF WOODS (D) - DEED DIMENSION

(S) - STORM SEWER MANHOLE S - SANITARY SEWER MANHOLE (M) - MEASURED DIMENSION = STORM SEWER BEEHIVE (P) - PLAT DIMENSION # - STORM SEWER INLET \varnothing – EXISTING UTILITY POLE CURB INLET S – SANITARY LATERAL ₩V – EXISTING WATER VALVE

🕀 – BENCHMARK W - EXISTING WATER METER 999.99 - SPOT ELEVATION ⊙ − EXISTING WATER LATERAL ♥ - EXISTING FIRE HYDRANT U - END SECTION

PROPOSED LEGEND:

—999— – PROPOSED CONTOUR

- PROPOSED STORM STRUCTURE NO.

- PROPOSED STORM INLET PROPOSED STORM END SECTION

999.9 - PROPOSED SPOT ELEVATION

999.99 - PROPOSED PAVEMENT ELEVATION - STORM SEWER OVERFLOW PATH

A – A.D.A. HANDICAP RAMP B - 2' ROLL CURB

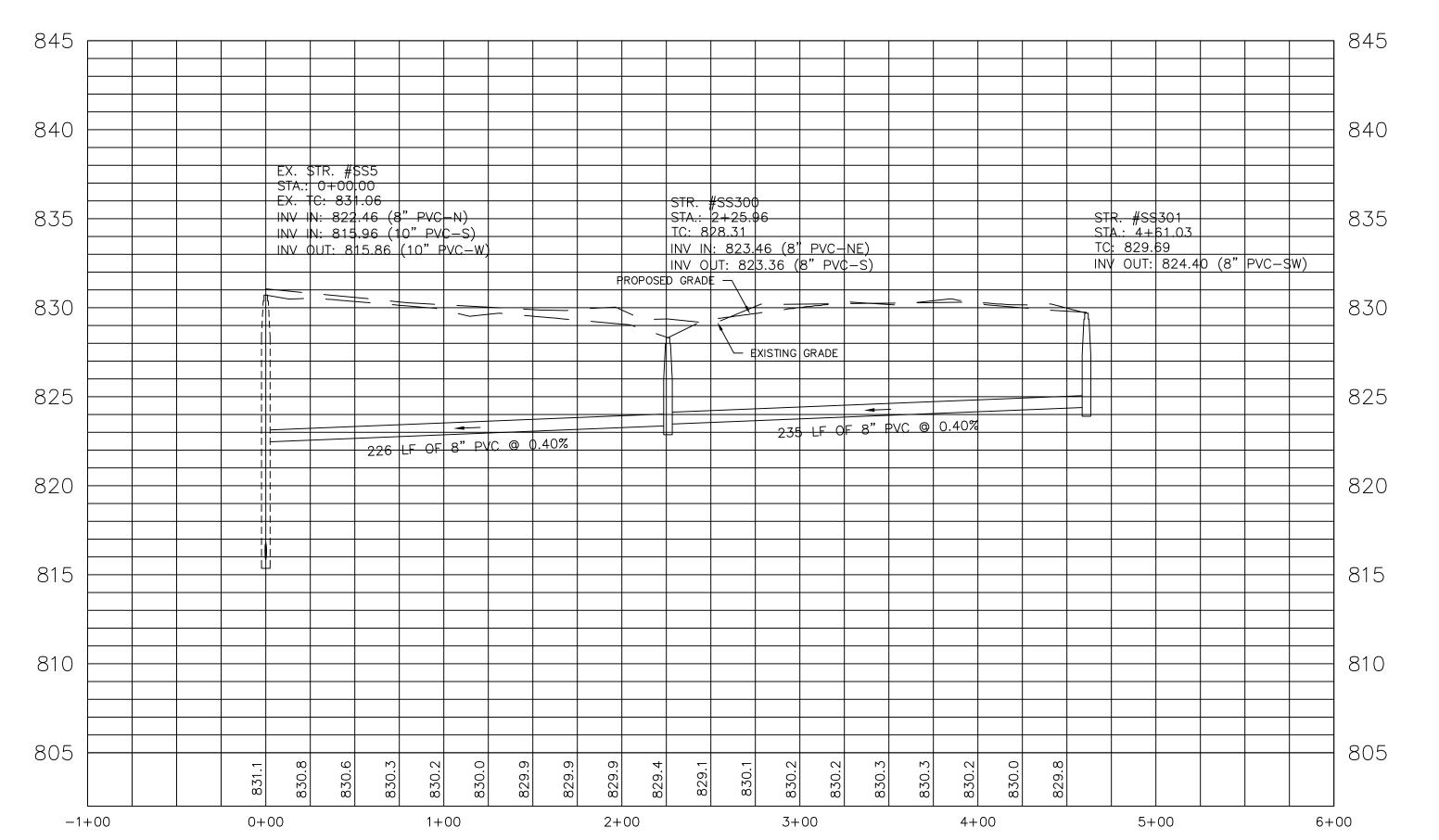
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EXISTING SANITARY MANHOLE EXISTING SANITARY MANHOLE EXISTING SANITARY MANHOLE EXISTING SANITARY MANHOLE T.C.=828.25 T.C.=827.84 T.C.=828.08 T.C.=829.65 S.E. INV.=813.28 (10" PVC) S. INV.=811.35 (10" PVC) S. INV.=812.54 (10" PVC) S.E. INV.=814.50 (10" PVC) N. INV.=812.98 (10" PVC) APPARENT E. INV.=815.03 N. INV.=812.49 (10" PVC) N.W. INV.=814.50 (10" PVC)

EXISTING SANITARY MANHOLE T.C.=831.06 N. INV.=822.46 (8" PVC) S. INV.=816.87 (10" PVC) N. DROP INV.=816.36 N.E. INV.=816.67 (10" PVC) N.W. INV.=815.86 (10" PVC) S.W. INV.=815.96 (10" PVC)

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PROFILE VIEW - SCALE: 1"=50' (H) 1"=5' (V)



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14.) The contractor shall contact all applicable utilities and verify any and all fees associated with the installation of all utilities.

15.) Follow all local and state codes in reference to domestic line installation and sanitary lateral

16.) A minimum of 18" vertical and 10' horizontal separation to be maintained between the water mains, hydrants and sanitary and storm sewers.

17.) Trenches under paved areas (excluding sidewalks) shall be backfilled with granular material

18.) The contractor shall field verify the location and the pipe invert depth where the proposed

13.) Contractor to provide pipe bedding and compact to 95% density of standard proctor in paved

installation.

and compacted in lifts. Granular material shall extend 5 feet beyond the limits of the paved area.

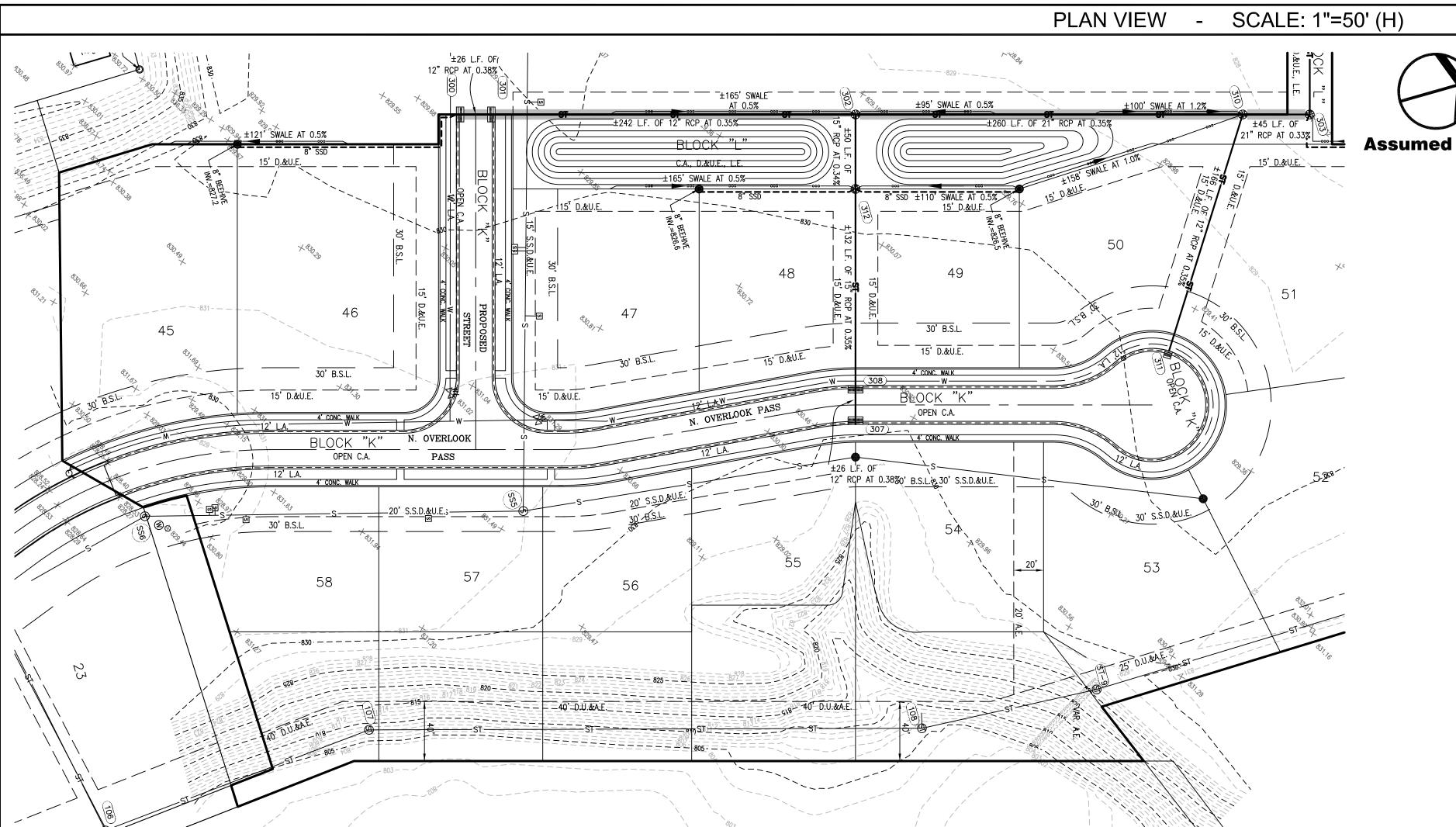
connection is made to the existing sanitary sewer.

Sheet Number

NOTES

1.) SEE TOWN OF CUMBERLAND DETAIL AND

SPECIFICATION SHEETS FOR SANITARY INFORMATION.



Assumed North

LEGEND:

OHE — OVERHEAD UTILITY LINE - UNDERGROUND WATER MAIN - APPROX. EDGE OF WOODS (D) - DEED DIMENSION (M) - MEASURED DIMENSION (P) - PLAT DIMENSION \varnothing - EXISTING UTILITY POLE

CURB INLET S - SANITARY LATERAL WY - EXISTING WATER VALVE 🕀 – BENCHMARK W - EXISTING WATER METER 99.99 - SPOT ELEVATION ⊙ − EXISTING WATER LATERAL

N. INV.=799.83 (30" N-12)

S. INV.=799.73 (30" N-12)

N. INV.=799.17 (30" N-12)

S. INV.=799.07 (30" N-12)

W. INV.=800.67

♥ - EXISTING FIRE HYDRANT U - END SECTION

- INDICATES 1/4 SECTION

(S) - STORM SEWER MANHOLE S - SANITARY SEWER MANHOLE

STORM SEWER BEEHIVE

- STORM SEWER INLET

PROPOSED LEGEND:

—● (SS#) - PROPOSED SANITARY MANHOLE

> - PROPOSED STORM STRUCTURE NO.

- PROPOSED STORM END SECTION

- PROPOSED SPOT ELEVATION - PROPOSED PAVEMENT ELEVATION

- STORM SEWER OVERFLOW PATH

(A) - A.D.A. HANDICAP RAMP

B - 2' ROLL CURB

EXISTING STORM SEWER STRUCTURE TABLE

ST-4	ST-6	(ST-7)	ST-8
EXISTING STORM MANHOLE T.C.=827.80 N. INV.=820.20 (18" RCP) E. INV.=820.10 (18" RCP)	EXISTING STORM INLET T.C.=823.95 W. INV.=819.45 (18" RCP) E. INV.=819.45 (18" RCP)	EXISTING STORM MANHOLE T.C.=827.21 W. INV.=819.01 (18" RCP) S. INV.=818.91 (18" RCP)	EXISTING STORM MANHOLE T.C.=831.75 N. INV.=818.65 (18" RCP) S. INV.=818.55 (18" RCP)
<u>(ST-9)</u>	(108)	(107)	(106)
EXISTING STORM MANHOLE T.C.=826.12 N. INV.=817.87 (18" RCP)	STORM MANHOLE PER PRIOR PROPOSED PLANS T.C.=806.50	STORM MANHOLE PER PRIOR PROPOSED PLANS T.C.=806.50	STORM MANHOLE PER PRIOR PROPOSED PLANS T.C.=804.50

T.C.=806.50 N. INV.=801.04 (18" RCP)

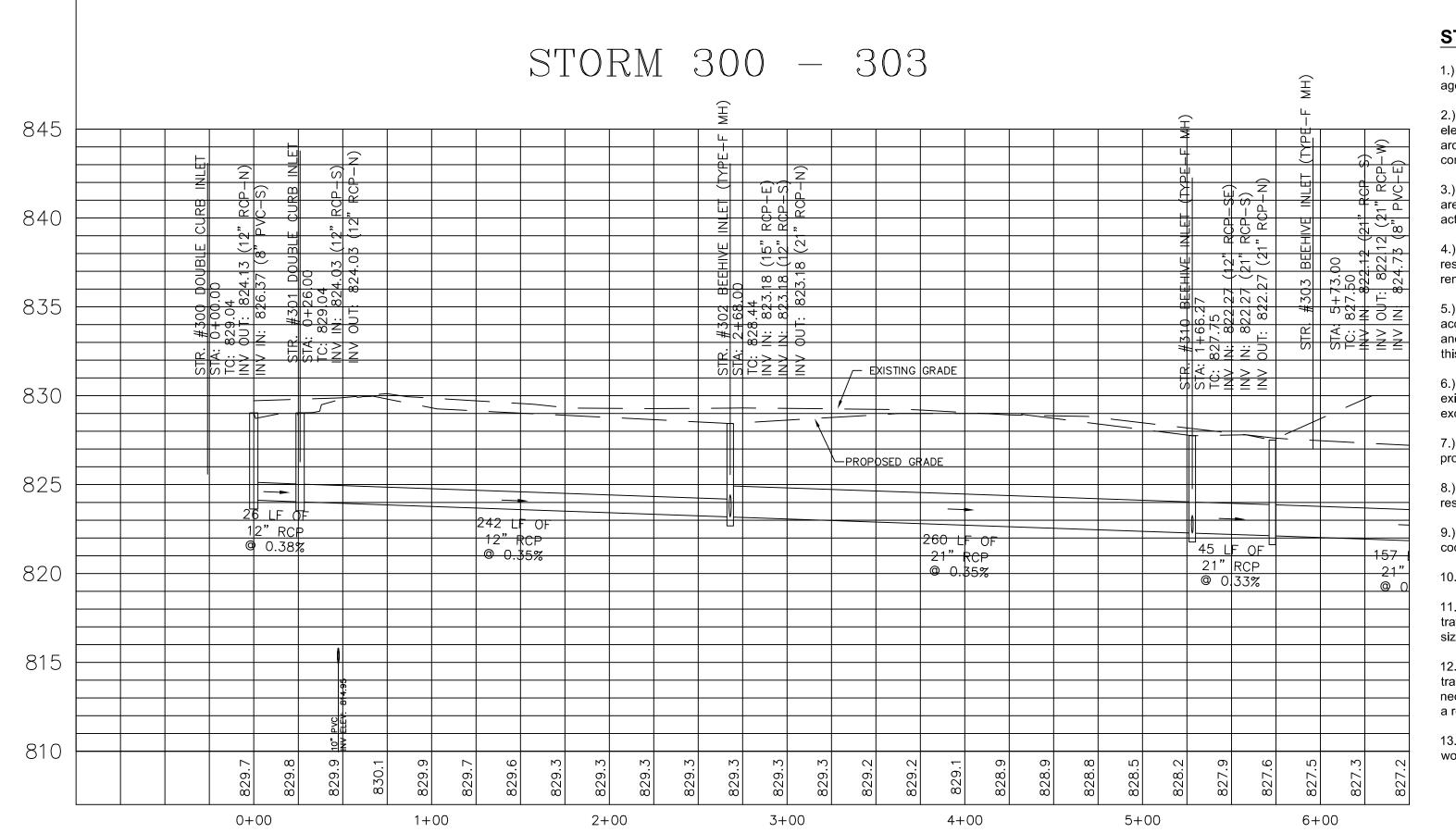
W. INV.=801.04 (18" RCP)

S. INV.=800.94 (30" N-12)



CALL BEFORE YOU DIG. Call 811 or 800-382-5544 Before you Dig!

PROFILE VIEW - SCALE: 1"=50' (H) 1"=5' (V)



STORM SEWER NOTES

S. INV.=809.87 (18" RCP)

1.) These documents are subject to periodic revisions per requests of the client and or governing agencies. It is the holder's responsibility to verify the most current issue prior to any use.

2.) Contractor shall recognize respective work and responsibility to verify location, size, and elevation of existing utilities, structures, pipes, pavements, etc. as related to their work. Notify architect of any conflict and/or discrepancies in the construction documents prior to the start of construction.

3.) The contractor shall be responsible for obtaining or verifying that all permits and approvals are obtained from the respective city, county, and state agencies prior to any construction

4.) It is the contractor's responsibility to acquaint himself with subsoil conditions. Contractor is responsible for all cut/fill materials required to match compacted design subgrade. Topsoil removal limits to be determined by the contractor and/or owner.

5.) The plans show the location of all known utilities located within the limits of the contract according to information provided by the various utility companies, previous construction plans and as evidenced by observation of above ground conditions by the surveyor. The accuracy of this information is not guaranteed.

6.) All pavement patching due to utilities installation, construction of curbs, etc., or damage to existing pavement during construction shall be patched with a pavement section which meets or exceeds the specifications by the governing agency or specifications by INDOT.

7.) All existing manholes, catch basin grates, water valves, gas valves, etc., shall be adjusted to proposed finish grade elevation.

8.) All pipe lengths shown on drawings are for hydraulic calculation purposes only. It is the responsibility of the contractor to determine exact lengths required for actual installation.

9.) Construction of all sewer lines and structures shall be in accordance with local and state code rules and regulations.

10.) Repair, replace or maintain all existing utilities within limits of construction.

11.) The contractor shall be responsible to provide at his expense all automobile and pedestrian traffic control devices required by federal, state, city or local agencies. The mount, location and size shall be per the direction of the governing agency.

12.) It is the responsibility of the contractor to remove all mud, dirt, gravel and other materials tracked onto any public or private streets or sidewalks. The contractor must clean these daily if necessary. The contractor must use water or other expectable methods to keep airborne dust to

13.) Coordinate all work with county, city or governing utility at least 48 hours prior to the start of

14.) Contractor to provide pipe bedding and compact to 95% density of standard proctor in paved areas. Pipe bedding in non-paved areas shall be compacted to 85% density of standard proctor.

15.) The contractor shall contact all applicable utilities and verify any and all fees associated with the installation of all utilities.

16.) A minimum of 18" vertical and 10' horizontal separation to be maintained between the water mains, hydrants and sanitary and storm sewers.

17.) Trenches under paved areas (excluding sidewalks) shall be backfilled with granular material

18.) The contractor shall field verify the location and the pipe invert depth where the proposed

19.) Verify existing inverts of storm sewer and adjust as required. Document any changes.

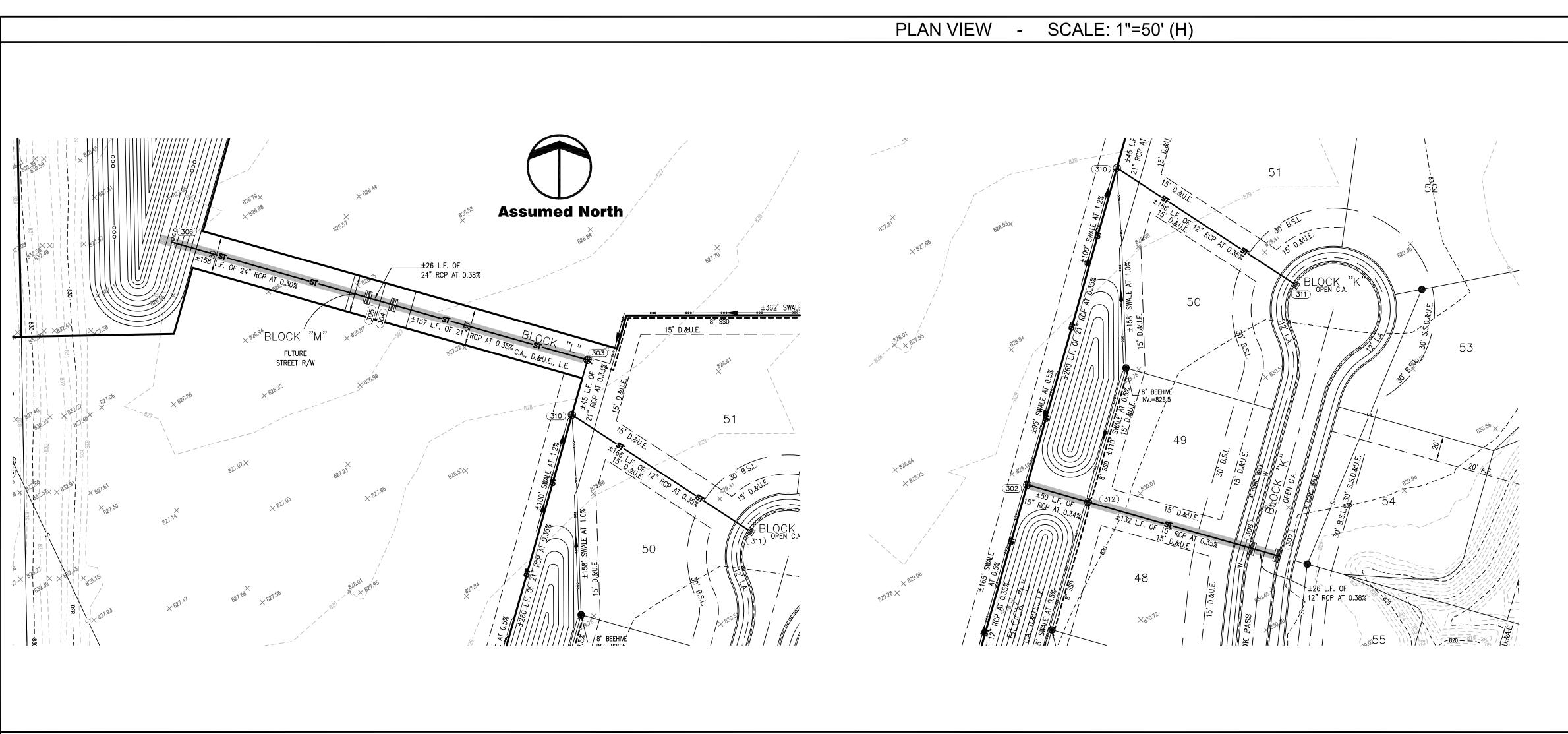
20.) Structures receiving sub-surface drain (SSD) shall have both ports core drilled. "T" or "Y"

and compacted in lifts. Granular material shall extend 5 feet beyond the limits of the paved area.

connection is made to the existing sewer.

blind connections are not allowed.

21.) All storm sewer castings shall be labeled "Dump No Waste - Drains To Waterway".



- OHE OVERHEAD UTILITY LINE
- APPROX. EDGE OF WOODS
 - (D) DEED DIMENSION (M) - MEASURED DIMENSION
 - (P) PLAT DIMENSION \varnothing - EXISTING UTILITY POLE
 - W EXISTING WATER VALVE (W) - EXISTING WATER METER ⊙ – EXISTING WATER LATERAL
 - $\sqrt{999}.99$ SPOT ELEVATION 💙 – EXISTING FIRE HYDRANT

O - REBAR/PIPE FOUND

- INDICATES 1/4 SECTION CORNER

S - STORM SEWER MANHOLE S - SANITARY SEWER MANHOLE

€ - STORM SEWER BEEHIVE # - STORM SEWER INLET

- CURB INLET

S - SANITARY LATERAL 🕀 – BENCHMARK

U - END SECTION

PROPOSED LEGEND:

——999— – PROPOSED CONTOUR

—● (SS#) - PROPOSED SANITARY MANHOLE ### - PROPOSED STORM STRUCTURE NO.

(- PROPOSED STORM END SECTION

999.9 - PROPOSED SPOT ELEVATION

999.99 - PROPOSED PAVEMENT ELEVATION

- STORM SEWER OVERFLOW PATH

A – A.D.A. HANDICAP RAMP

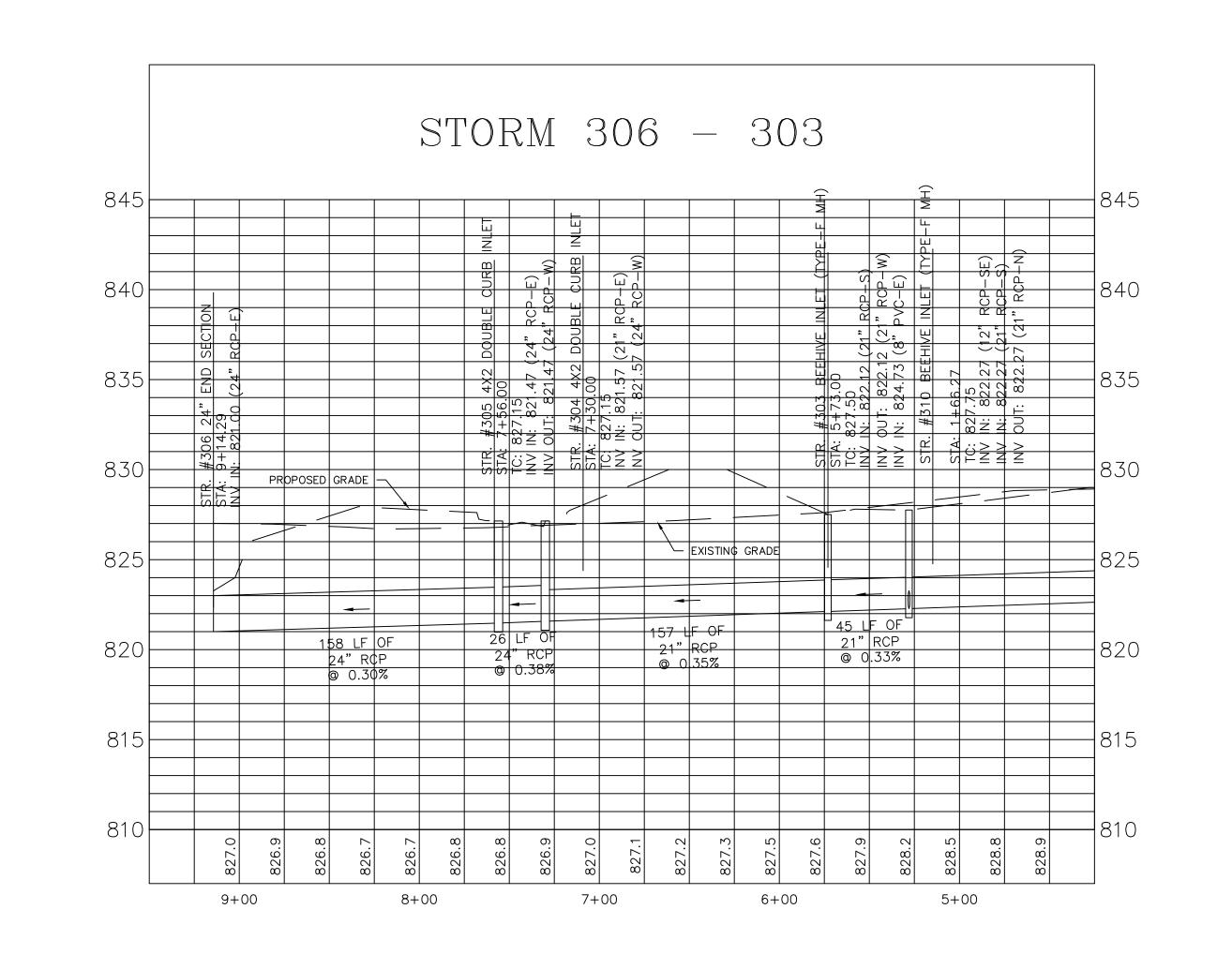
B - 2' ROLL CURB

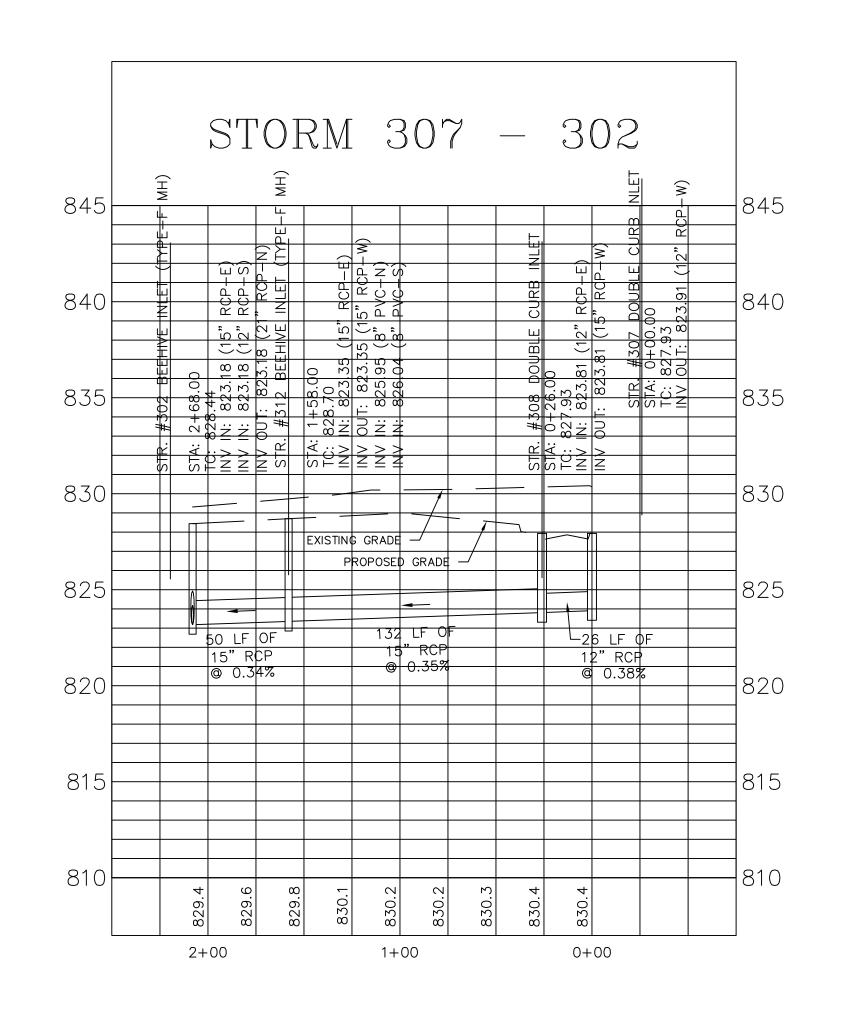


CALL BEFORE YOU DIG.

Call 811 or 800-382-5544 Before you Dig!

PROFILE VIEW SCALE: 1"=50' (H) 1"=5' (V)





EXISTING STORM SEWER STRUCTURE TABLE:

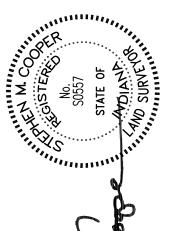
91 +	91 0	91 1)	91 0
EXISTING STORM MANHOLE T.C.=827.80 N. INV.=820.20 (18" RCP) E. INV.=820.10 (18" RCP)	EXISTING STORM INLET T.C.=823.95 W. INV.=819.45 (18" RCP) E. INV.=819.45 (18" RCP)	EXISTING STORM MANHOLE T.C.=827.21 W. INV.=819.01 (18" RCP) S. INV.=818.91 (18" RCP)	EXISTING STORM MANHOLE T.C.=831.75 N. INV.=818.65 (18" RCP) S. INV.=818.55 (18" RCP)

ST-9	108	(107)	(106)
EXISTING STORM MANHOLE	STORM MANHOLE PER	STORM MANHOLE PER	STORM MANHOLE PER
T.C.=826.12	PRIOR PROPOSED PLANS	PRIOR PROPOSED PLANS	PRIOR PROPOSED PLANS
N. INV.=817.87 (18" RCP)	T.C.=806.50	T.C.=806.50	T.C.=804.50
S. INV.=809.87 (18" RCP)	N. INV.=801.04 (18" RCP)	N. INV.=799.83 (30" N-12)	N. INV.=799.17 (30" N-12)
,	W. INV.=801.04 (18" RCP)	S. INV.=799.73 (30" N-12)	S. INV.=799.07 (30" N-12)
	S. INV.=800.94 (30" N-12)	,	W. INV.=800.67

STORM SEWER NOTES

- See Sheet C7.0 for all storm sewer notes.





Assumed North

LEGEND:

- OHE - OVERHEAD UTILITY LINE

----- - UNDERGROUND WATER MAIN - APPROX. EDGE OF WOODS

(D) - DEED DIMENSION

(M) - MEASURED DIMENSION (P) - PLAT DIMENSION

 \varnothing - EXISTING UTILITY POLE WY - EXISTING WATER VALVE W - EXISTING WATER METER ⊙ − EXISTING WATER LATERAL 😙 – EXISTING FIRE HYDRANT

U - END SECTION

O - REBAR/PIPE FOUND - INDICATES 1/4 SECTION CORNER

⑤ - STORM SEWER MANHOLE S - SANITARY SEWER MANHOLE STORM SEWER BEEHIVE

- CURB INLET S - SANITARY LATERAL

■ - STORM SEWER INLET

BENCHMARK y999.99 - SPOT ELEVATION

PROPOSED LEGEND:

——999— - PROPOSED CONTOUR

- SS#) - PROPOSED SANITARY MANHOLE ### - PROPOSED STORM STRUCTURE NO.

→ PROPOSED STORM INLET

 PROPOSED STORM END SECTION 999.9 - PROPOSED SPOT ELEVATION

999.99 - PROPOSED PAVEMENT ELEVATION - STORM SEWER OVERFLOW PATH

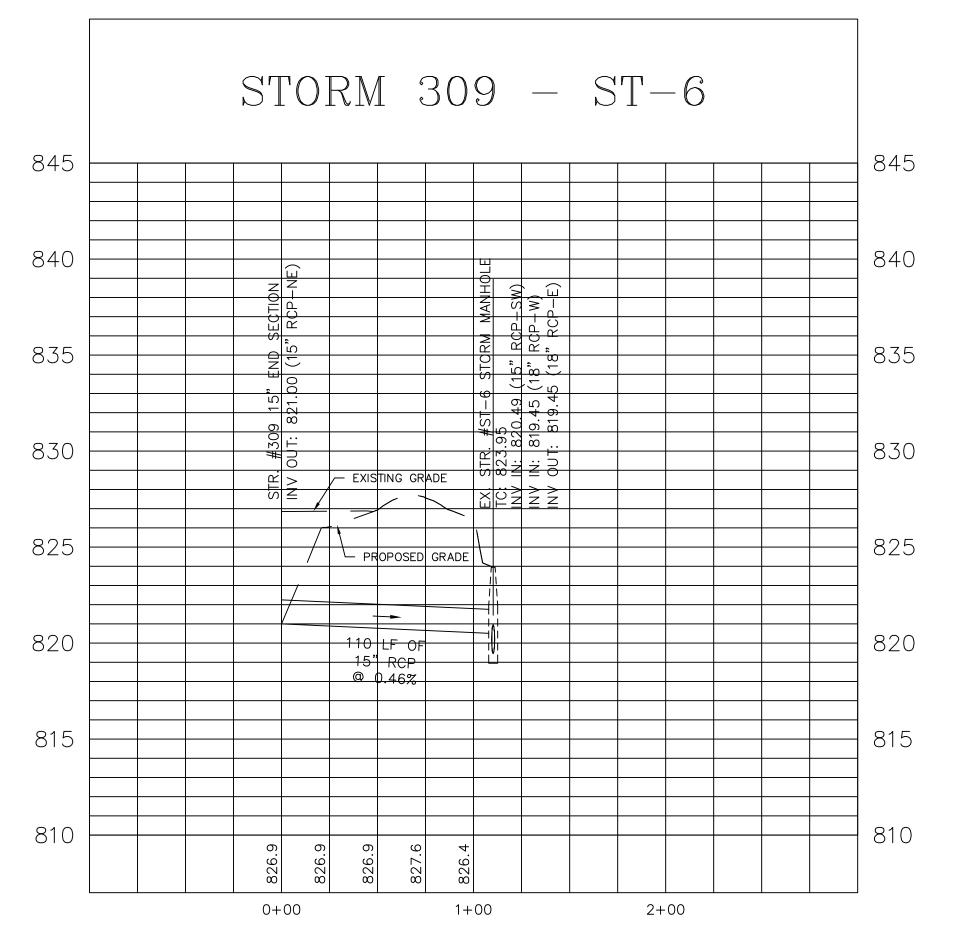
A – A.D.A. HANDICAP RAMP

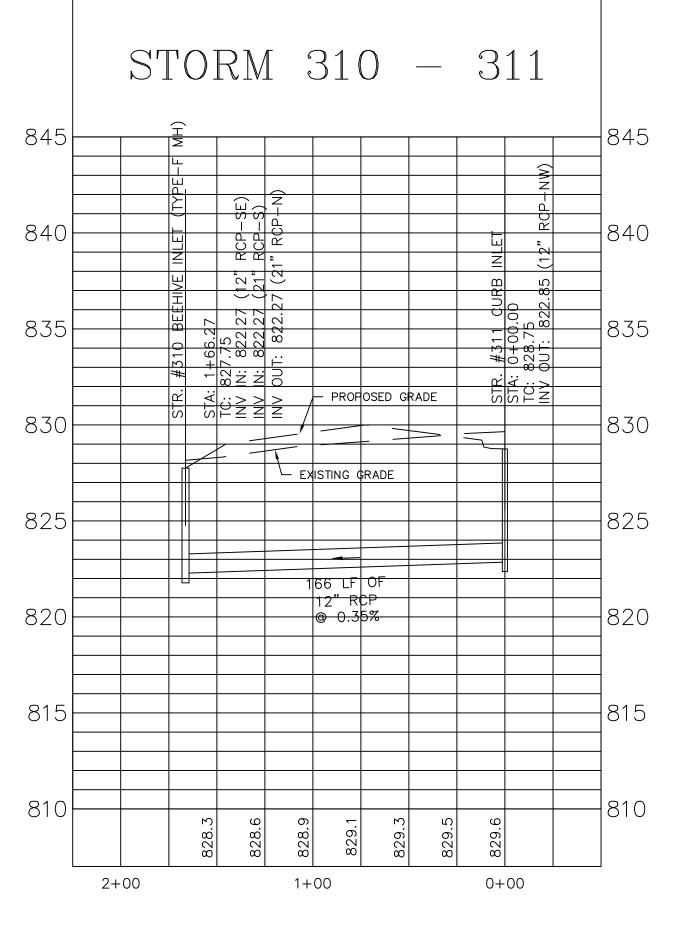
B - 2' ROLL CURB



Call 811 or 800-382-5544 Before you Dig!

SCALE: 1"=50' (H) 1"=5' (V) PROFILE VIEW





±362' SWALE AT 1.0%

51

8" BEEHIVE INV.=826.9

EXISTING STORM SEWER STRUCTURE TABLE:

ST-6

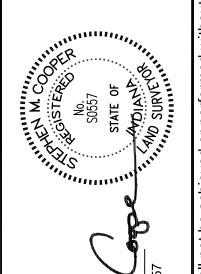
ST-4

EXISTING STORM MANHOLE T.C.=827.80 N. INV.=820.20 (18" RCP) E. INV.=820.10 (18" RCP)	EXISTING STORM INLET T.C.=823.95 W. INV.=819.45 (18" RCP) E. INV.=819.45 (18" RCP)	EXISTING STORM MANHOLE T.C.=827.21 W. INV.=819.01 (18" RCP) S. INV.=818.91 (18" RCP)	EXISTING STORM MANHOLE T.C.=831.75 N. INV.=818.65 (18" RCP) S. INV.=818.55 (18" RCP)
(ST-9) EXISTING STORM MANHOLE T.C.=826.12 N. INV.=817.87 (18" RCP)	(108) STORM MANHOLE PER PRIOR PROPOSED PLANS T.C.=806.50	(107) STORM MANHOLE PER PRIOR PROPOSED PLANS T.C.=806.50	(106) STORM MANHOLE PER PRIOR PROPOSED PLANS T.C.=804.50
S. INV.=809.87 (18" RCP)	N. INV.=801.04 (18" RCP) W. INV.=801.04 (18" RCP) S. INV.=800.94 (30" N-12)	N. INV.=799.83 (30" N-12) S. INV.=799.73 (30" N-12)	N. INV.=799.17 (30" N-12) S. INV.=799.07 (30" N-12) W. INV.=800.67

STORM SEWER NOTES

- See Sheet C7.0 for all storm sewer notes.





- UNDERGROUND WATER MAIN
- APPROX. EDGE OF WOODS

(D) - DEED DIMENSION
 (M) - MEASURED DIMENSION
 (P) - PLAT DIMENSION
 Ø - EXISTING UTILITY POLE

₩ – EXISTING WATER VALVE
₩ – EXISTING WATER METER

 \odot - EXISTING WATER LATERAL $\times 999.99$ - SPOT ELEVATION $\times 999.99$ - EXISTING CONTOUR

→ END SECTION

- "MAG" NAIL - REBAR/PIPE FOUND

REBAR/PIPE FOUNDINDICATES 1/4 SECTION CORNER

⑤ - STORM SEWER MANHOLE
 ⑤ - SANITARY SEWER MANHOLE
 ⑥ - STORM SEWER BEEHIVE
 Ⅲ - STORM SEWER INLET
 Ⅲ - CURB INLET
 ⑤ - SANITARY LATERAL

 \bullet – BENCHMARK $\times 9^{99}$ 99 – SPOT ELEVATION

Assumed North

15 30 60

Scale: 1" = 30'



call 811 or 800-382-5544 Before you Dig!

PROPOSED LEGEND:

—999— - PROPOSED CONTOUR

- SS# - PROPOSED SANITARY MANHOLE

PROPOSED STORM STRUCTURE NO.PROPOSED STORM INLET

- PROPOSED STORM END SECTION999.9 - PROPOSED SPOT ELEVATION

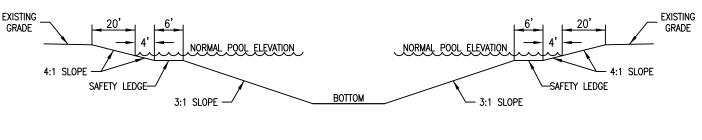
999.99 − PROPOSED PAVEMENT ELEVATION→ STORM SEWER OVERFLOW PATH

A – A.D.A. HANDICAP RAMP
B – 2' ROLL CURB

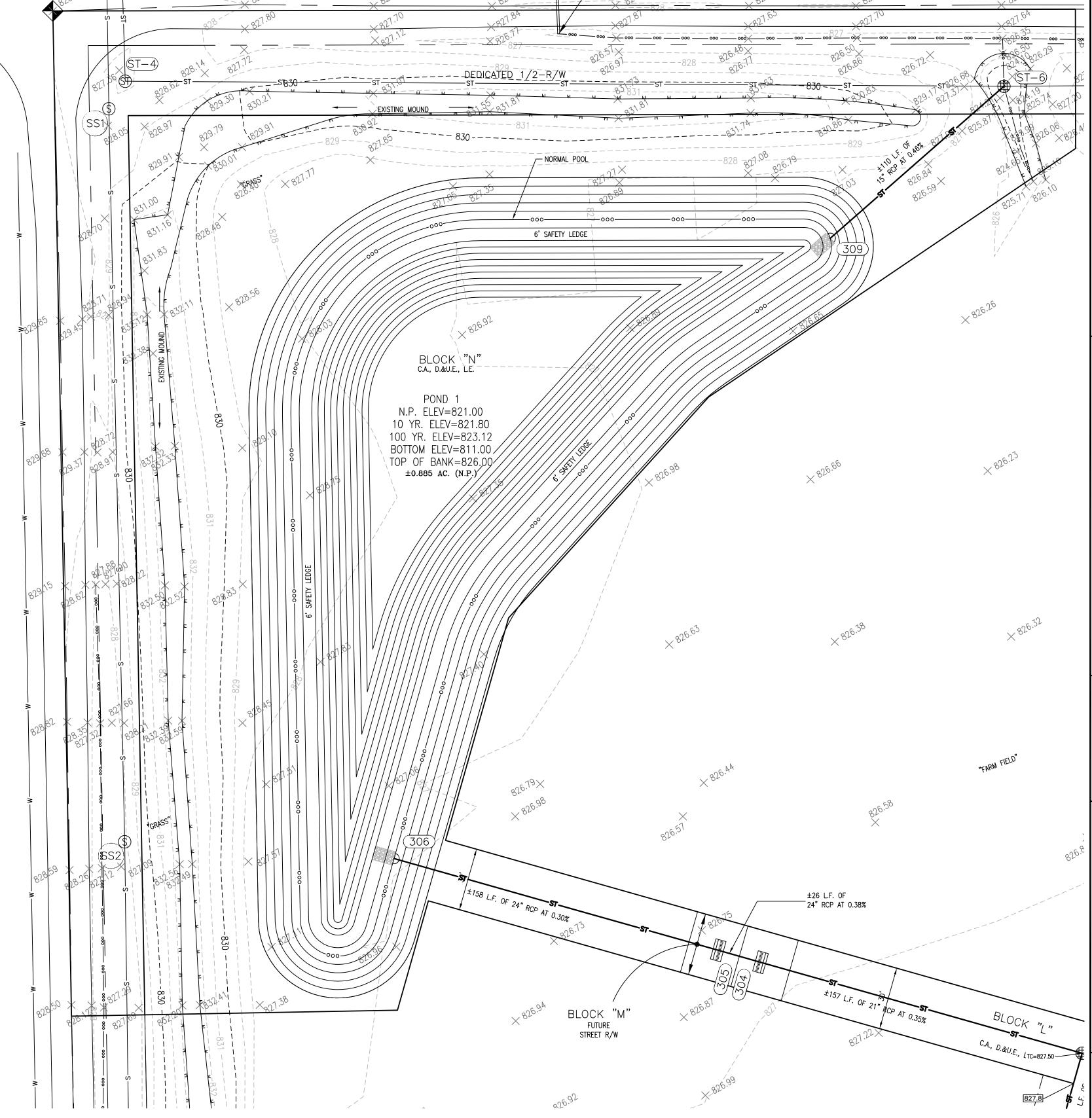
EXISTING STORM SEWER STRUCTURE TABLE:

<u>(ST-4)</u>	(ST-6)	(ST-7)	<u>(\$1–8)</u>
EXISTING STORM MANHOLE	EXISTING STORM INLET	EXISTING STORM MANHOLE	EXISTING STORM MANHOLE
T.C.=827.80	T.C.=823.95	T.C.=827.21	T.C.=831.75
N. INV.=820.20 (18" RCP)	W. INV.=819.45 (18" RCP)	W. INV.=819.01 (18" RCP)	N. INV.=818.65 (18" RCP)
E. INV.=820.10 (18" RCP)	E. INV.=819.45 (18" RCP)	S. INV.=818.91 (18" RCP)	S. INV.=818.55 (18" RCP)

ST-9	108	107	106
EXISTING STORM MANHOLE T.C.=826.12 N. INV.=817.87 (18" RCP) S. INV.=809.87 (18" RCP)	STORM MANHOLE PER PRIOR PROPOSED PLANS T.C.=806.50 N. INV.=801.04 (18" RCP) W. INV.=801.04 (18" RCP) S. INV.=800.94 (30" N-12)	STORM MANHOLE PER PRIOR PROPOSED PLANS T.C.=806.50 N. INV.=799.83 (30" N-12) S. INV.=799.73 (30" N-12)	STORM MANHOLE PER PRIOR PROPOSED PLAN T.C.=804.50 N. INV.=799.17 (30" N- S. INV.=799.07 (30" N- W. INV.=800.67



DETENTION BASIN CROSS-SECTION
NO SCALE

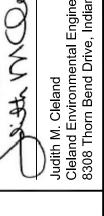


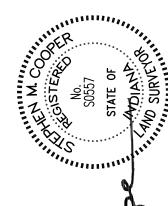
STORM SEWER NOTES

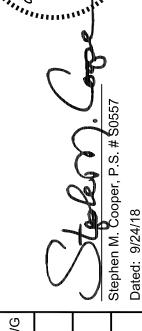
- See Sheet C7.0 for all storm sewer notes.



and horive, Indianapolis, IN 46278







C FILE #: 2017-124.BASE.DWG
DATE: 9/24/18
APPROVED BY: SMC
Stephen
DRAWN BY: MTL

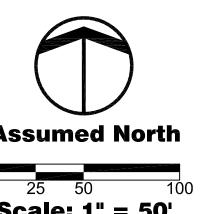
red For:
HANCOCK LAND CO., LLC
st Location:
PART OF W. 1/2 - N.E. 1/4
S 20 - T 15 N - R 06 E
SUGAR CREEK TOWNSHIP
HANCOCK COUNTY, INDIANA

onsulting & land services corporation

303 West Main Street, Knightstown, Indiana 46148
3)345-5943 Toll Free:(888)593-2667 Fax:(765)345-5692

Sheet Number

C7.3



ned N	lorth	
50 •: 1" =	100 50'	
	50	ned North 50 100 100 11" = 50'

——OHE———	-	OVERHEAD UTILITY LINE
s	_	SANITARY SEWER LINE
ST	_	STORM SEWER LINE
W	_	UNDERGROUND WATER MAIN
·····	-	APPROX. EDGE OF WOODS
(5)		DEED DIMENSION

	 OVERHEAD UTILITY LINE
	- SANITARY SEWER LINE
	- STORM SEWER LINE
	- UNDERGROUND WATER MAIN
~~	- APPROX. EDGE OF WOODS
(D)	DEED DIMENSION

- OVERHEAD UTILITY LINE	— "MAG" NAIL
- SANITARY SEWER LINE	O - REBAR/PIPE FOUND
- STORM SEWER LINE	- INDICATES 1/4 SECTION
- UNDERGROUND WATER MAIN	- INDICATES 1/4 SECTION CORNER
- APPROX. EDGE OF WOODS	🕥 - STORM SEWER MANHOLE
- DEED DIMENSION	S - SANITARY SEWER MANHOLE

(M) - MEASURED DIMENSION (P) - PLAT DIMENSION \emptyset - EXISTING UTILITY POLE

,-			
₩ -	EXISTING	WATER	VALVE
W -	EXISTING	WATER	METER
⊙ -	EXISTING	WATER	LATERAL

STORM SEWER BEEHIVE ■ - STORM SEWER INLET - CURB INLET S - SANITARY LATERAL

- BENCHMARK

- EXISTING WATER LATERAL	$ imes_{gg}$. g – SPOT ELEVATION
- EXISTING FIRE HYDRANT	
- END SECTION	

LEGEND:



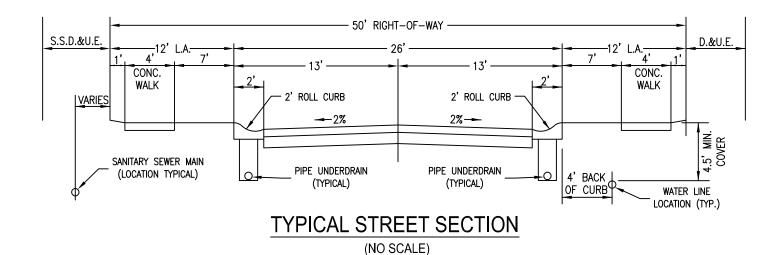
Call 811 or 800-382-5544 Before you Dig!

PROPOSED LEGEND:

- PROPOSED SANITARY MANHOLE - PROPOSED STORM STRUCTURE NO. - PROPOSED STORM INLET PROPOSED STORM END SECTION

> - PROPOSED STORM END SECTION - PROPOSED STORM END SECTION

— → PROPOSED STORM END SECTION



GENERAL PLAN NOTES

1.) These documents are subject to periodic revisions per requests of the client and or governing agencies. It is the holder's responsibility to verify the most current issue prior to any use.

2.) Contractor shall recognize respective work and responsibility to verify location, size, and elevation of existing utilities, structures, pipes, pavements, etc. as related to their work. Notify architect/engineer of any conflict and/or discrepancies in the construction documents prior to the start of construction.

3.) The contractor shall be responsible for obtaining or verifying that all permits and approvals are obtained from the respective city, county, and state agencies prior to any construction activities.

4.) It is the contractor's responsibility to acquaint himself with subsoil conditions. Contractor is responsible for all cut/fill materials required to match compacted design subgrade. Topsoil removal limits to be determined by the contractor and/or owner.

5.) The plans show the location of all known utilities located within the limits of the contract according to information provided by the various utility companies, previous construction plans and as evidenced by observation of above ground conditions by the surveyor. The accuracy of this information is not guaranteed.

6.) All pavement patching due to utilities installation, construction of curbs, etc., or damage to existing pavement during construction shall be patched with a pavement section which meets or exceeds the specifications by the city or specifications by INDOT.

7.) All existing manholes, catch basin grates, water valves, gas valves, etc., shall be adjusted to proposed finish grade elevation.

8.) All pipe lengths shown on drawings are for hydraulic calculation purposes only. It is the responsibility of the contractor to determine exact lengths required for actual installation.

9.) Repair, replace or maintain any/all existing utilities within limits of construction.

10.) Coordinate all work with county, city or governing utility at least 48 hours prior to the start of

11.) All construction on this site to be performed in compliance with O.S.H.A. standards for worker safety.

12.) The proposed elevations depicted hereon are based upon existing surface conditions as of the date of the within survey. No information about fluctuating water tables, soil conditions, or soil types within the subject tract has been provided by client or others unless otherwise noted. If any groundwater is witnessed during the construction process, Coor Consulting & Land Services, Corp. should be notified immediately and additional construction techniques may be incorporated to allocate future problems.

13.) Fill material shall consist of earth obtained from cut areas, borrow pits or other approved sources. Earth shall be free from organic matter and other deleterious substances and large rocks. The fill material shall be placed in layers not to exceed six inches following compaction, proper moisture content of fill material will be such to achieve specified compaction density. All fill beneath paved areas, floor slabs and future buildings shall be compacted to at least 95% of the maximum dry density per ASTM D-1557. Field compacting test shall be run on each lift, in fill sections, and the required compaction on each lift shall be in accordance with INDOT Section

14.) Follow all local and state codes in reference to domestic line installation and sanitary lateral

15.) All water mains to be 54" minimum below grade.

16.) The contractor shall contact all applicable utilities and verify any and all fees associated with the installation of all utilities.

17.) Contractor shall coordinate with water company and architectural plans for fire protection line and domestic service line installation.

18.) Contractor to provide pipe bedding and compact to 95% density of standard proctor in paved areas. Pipe bedding in non-paved areas shall be compacted to 85% density of standard proctor.

19.) Maintain a minimum of 10' horizontal clearance between sanitary lateral line and domestic

20.) All construction specifications should comply with current specifications.

21.) Trenches under paved areas (excluding sidewalks) shall be backfilled with granular material

and compacted in lifts. Granular material shall extend 5 feet beyond the limits of the paved area.

22.) See Plat for lot dimensions, building lines, and easements.

23.) No foundry sand is allowed for any construction purposes, per Hancock County ordinance.

WATER PLAN NOTES

1.) Town of Cumberland Water System Construction Standards, Specifications and Details were used for existing water main construction and are to be used for all new water main and service

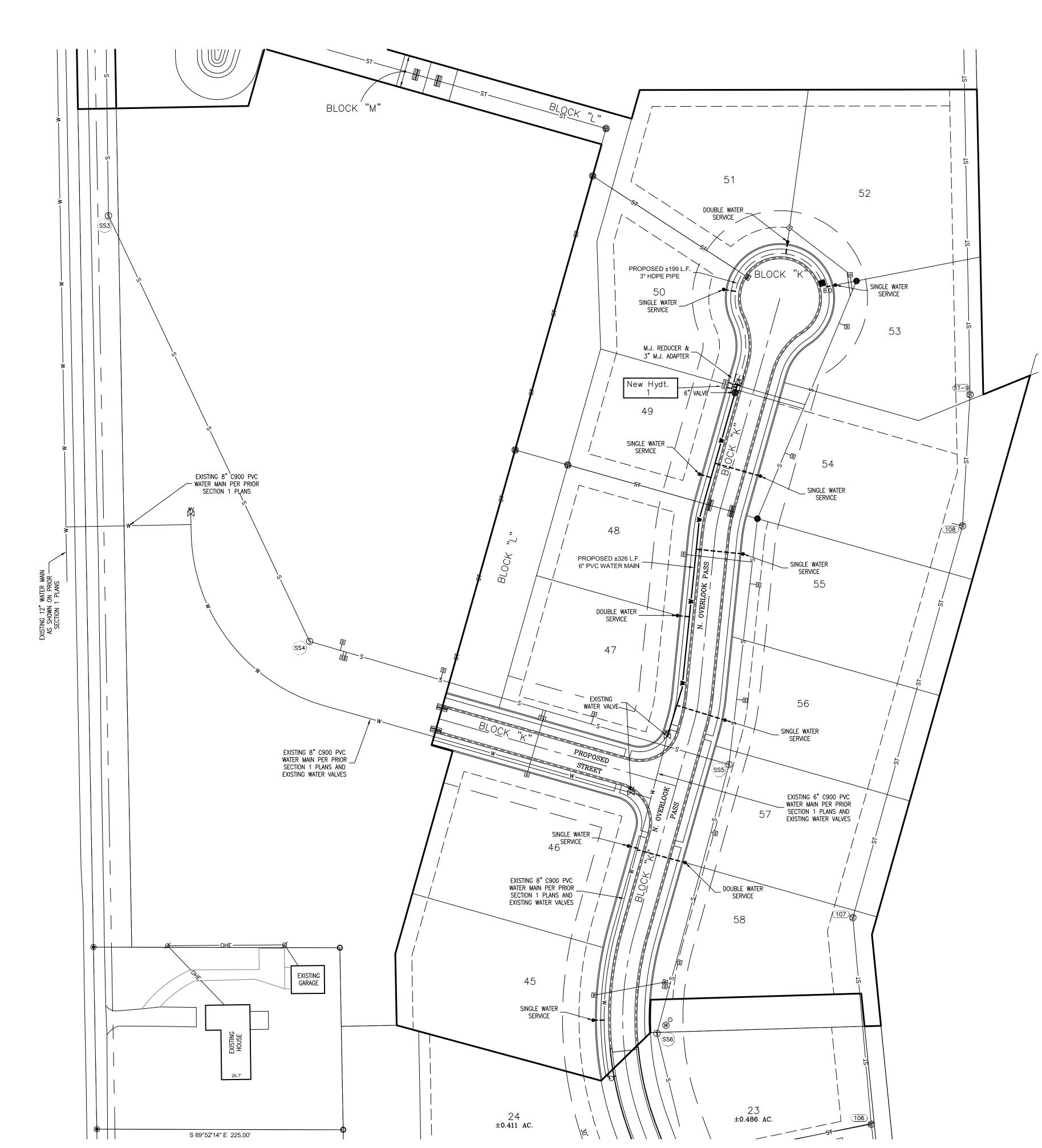
2.) The material of the existing water main shall be verified and if PVC or HDPE, saddles shall be provided for all water service connections.

3.) Any "live taps" are to be coordinated with Town of Cumberland Water Department.

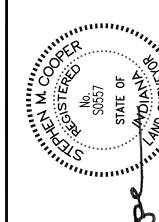
4.) Adjust hydrants so the centerline of hydrant outlet nozzles are not less that 18 inches nor more than 20 inches above finished grade. Provide hydrant extensions where required to obtain proper elevation.

6.) Service line shall be HDPE or Type "K" copper per Section 02661 of the Town's Standards. Diameter of service line shall be 1-inch for both single meter and double meter locations.

7.) Service line installation shall be per Section 02661 of the Town's Standards.







ASSESSMENT OF CONSTRUCTION PLAN ELEMENTS (SECTION A):

A1) Index showing locations of required Plan Elements: See Erosion Control Plan

A2) 11-inch by 17-inch plat showing building lot numbers/boundaries and road layout/names: Attached. A3) Narrative describing the nature and purpose of the project: The within project consists of the proposed future sections of "The Overlook" and is based upon the recorded plats of "The Overlook Section One" and "The Overlook Section Two" as referenced hereon. This project includes proposed residences, streets, curbs, walks, ponds, storm sewers, sanitary sewers and all other related underground utilities. See Site, Grading and Utility Plans.

A4) Vicinity map showing project location: See Title Sheet. A5) Legal Description of the Project Site (Include Latitude and Longitude): See Title sheet for Land Description. Latitude (- 39° 44' 30" N) Longitude (- 85° 49' 30" N) L

A6) Location of all lots and proposed site improvements: See Development Plan.

A7) Hydrologic unit code: 0512020406005

A8) Notation of any State or Federal water quality permits: None Required.

A9) Specific points where storm water discharge will leave the site: Storm water will leave the site via the proposed storm pipe connection between the proposed pond to the existing storm sewer line along the North side of the project. See Development Plan.

A10) Location and name of all wetlands, lakes, and watercourses on and adjacent to the site: Sugar Creek exists along the East side of the plat of "The Overlook Section One" located Southeasterly of the overall proposed site. See Development Plan.

A11) Identification of all receiving waters: Sugar Creek. A12) Identification of potential discharges to ground water: None.

A13) 100-year floodplains, floodways, and floodway fringes: The overall subject site is located within Zone AE and X per FIRM #18059C0207E, dated 3/17/14. See Development Plan

A14) Pre-construction and post-construction estimate of Peak Discharge: Reference Drainage Calculations.

A15) Adjacent land use, including upstream watershed: Residential. See Development Plan and Erosion Control Plan.

A16) Locations and approximate boundaries of all disturbed areas: See Erosion Control Plan. A17) Identification of existing vegetative cover: See Existing Conditions Plan and Erosion Control Plan.

A18) Soils map including soil descriptions and limitations: See Erosion Control Plan. A19) Locations, size and dimensions of proposed storm water systems: See Development Plan and Storm Sewer Plan & Profile.

A20) Plans for any off-site construction activities associated with this project: Temporary construction entrance as shown hereon. A21) Locations of proposed soil stockpiles and/or borrow/disposal areas: See Erosion Control Plan

A22) Existing site topography at an interval appropriate to indicate drainage patterns: See Existing Conditions Plan or Erosion Control Plan. A23) Proposed final topography at an interval appropriate to indicate drainage patterns: See Development Plan.

ASSESSMENT OF STORMWATER POLLUTION PREVENTION PLAN (SECTIONS B & C):

B1) Description of potential pollutant sources associated with construction activities: The possibility of leaking material or fuel storage areas, or a leaking vehicle or equipment, etc. from construction activities exists.

B2) Sequence describing stormwater quality measure implementation relative to land disturbing activities; See Erosion Control Plan.

B3) Stable construction entrance locations and specifications: See Erosion Control Plan. B4) Sediment control measures for sheet flow areas: Silt fence to be installed as depicted on the Erosion Control Plan.

B5) Sediment control measures for concentrated flow areas: See Erosion Control Plan. B6) Storm sewer inlet protection measure locations and specifications; Inlet protection to be installed as depicted on the Erosion Control Plan.

B7) Runoff control measures: See Erosion Control Plan.

B8) Storm water outlet protection specifications: See Erosion Control Plan B9) Grade stabilization structure locations and specifications: See Erosion Control Plan.

B10) Location, dimensions, specifications, and construction details of each stormwater quality measure: See Erosion Control Plan and/or Detail Sheet. B11) Temporary surface stabilization methods appropriate for each season: See Figure 5.3 & 5.4 on Erosion Conrol Plan and/or Detail Sheet. B12) Permanent surface stabilization specifications: See Figure 5.2 & 5.4 on Erosion Control Plan and/or Detail Sheet.

B13) Material handling and spill prevention plan: Contact the Environmental Management Department (888) 233-7745 at any time immediately after an emergency spill occurs. Contact the owner of the contaminated property. Review the Material handling and spill prevention plan on the Erosion Control

B14) Monitoring and maintenance guidelines for each proposed stormwater quality measure: See Erosion Control Plan. B15) Erosion & sediment control specifications for individual building lots: See Erosion Control Plan.

C1) Description of pollutants and their sources associated with the proposed land use: The possibility exists of stormwater contamination from vehicular discharges or other hydrocarbons, grit (sediment) from wearing of road surfaces and falling or washing off of vehicles, trash from littering, and other improper disposals or storage.

C2) Sequence describing stormwater quality measure implementation: See Erosion Control Plan.

C3) Description of proposed post construction stormwater quality measures: See Erosion Control Plan. The grass will help filter any potential pollution. The swales, storm inlets, and pavement will be kept clean of all trash, leaves, and debris at all times.

C4) Location, dimensions, specifications, and construction details of each stormwater quality measure: See Erosion Control Plan. C5) Description of maintenance guidelines for post construction stormwater quality measures: All swales, storm inlets and pavement will be kept clean of all debris and trash at all times. All swales will be maintained with grass cover and will be replanted if required.

GENERAL NOTES:

1) These documents are subject to periodic revisions per requests of the client and or governing agencies. It is the holder's responsibility to verify the most current issue prior to any use. 2) All methods of controlling the erosion of soils and sediment on this site shall be performed in accordance with the City/Town, Indiana Department of

Environmental Management, and Indiana Department of Natural Resources requirements. 3) All land alterations, which strip the land of existing vegetation, including but not limited to regrading of a specific area, shall be done in such as way as to

minimize erosion. Whenever feasible, natural or existing vegetation shall be retained, protected and supplemented. 4) The duration of time in which an area is left exposed shall be kept to a practical minimum. The area should be stabilized as quickly as feasibly possible.

5) Temporary vegetation or other erosion control methods shall be used to protect exposed areas during development. 6) All permanent erosion control measures, final vegetation, and structural erosion control devices shall be installed as soon as practical under development

7) Sediment in water run off shall be trapped by the use of approved methods such as debris basins, silt fence, and siltration sumps until the disturbed area is stabilized. These methods shall be in place prior to any land alterations commence.

8) All installed trapping devices to be periodically inspected during dry periods and after each rainfall by site general contractor for performance of intended function. Any devices determined incapable of performing intended function shall be replaced.

9) Site Contractor shall maintain erosion control measures and devices as required throughout construction until a vegetative cover has established sufficiently to prevent erosion of the soils 10) The site contractor shall have available on site a water source capable of applying water to dry exposed soil to prevent wind erosion. The application of

water shall be at a rate that does not create surface run off. 11) All constructed grades at boundary lines shall match with the existing grades prior to construction.

12) During all phases of construction of this site, the General Contractor, Site Contractor, Owner, and any and all Sub-Contractors shall practice measures to t the erosion of soils due to the actions of water and wind. All erosion and se

13) It will be the responsibility of each contractor to verify the existence and condition of all utilities (aerial and subterranean) prior to and pertaining to their phase of the construction project. Any existing utilities or drain tiles damaged during construction need to be replaced, repaired, or incorporated into the

14) It shall be the contractors responsibility to contact the respective utility companies representatives for the proper locations of proposed utilities prior to construction or installation. The contractor shall notify in writing to the site designer and owner of any changes, omissions, or errors found within these documents or in the field before work is started or resumed

15) Per Indiana State Law (IS-69-1991), it is against the law to excavate without notification 2 working days prior to commencing work. 16) Unless otherwise noted, the Information pertaining to the size, location and disposition of existing utilities are as provided by the respective utility

companies. Prior to any construction all utility companies should be contacted for field location of utility services. 17) During construction activity, a lineal straw bale dam (or equivalent) shall be placed across any portion of the property where storm drainage exits the

18) At the time construction is to begin, the driveway shall be constructed and a gravel base shall be installed from the road to the work site to prevent carrying dirt onto the roadways. Any dirt deposited on the roadway must be removed by hand methods, such as shovel or broom, the same day. Flushing the surface with water will not be allowed.

19) All storm sewer structures must be prestamped with appropriate "Clean Water" message.

IMPLEMENTATION SCHEDULE: 1) The contractor shall schedule a pre-construction meeting with the City/Town prior to any constructing on the site being started. The contractor and/or

developer shall notify IDEM and the City/Town 48 hours prior to start of construction. 2) Following the required pre-construction meeting the contractor shall construct the "posting information center" on site. The contractor shall proceed to construct the construction entrance after all posting requirements have been met.

3) Immediately following the installation of the construction entrance, the contractor shall construct the maintenance and refueling area. 4) After the maintenance and refueling area is constructed, the contractor shall construct the concrete washout area per the specifications on the Specifications/Details Sheet.

5) Prior to any earth moving or demolition, the contractor shall install all silt fences as shown on the Erosion Control Plan. The details and specifications for silt fence are located on the Erosion Control Sheet and/or Detail Sheet.

6) The contractor shall protect all existing inlets with inlet protection as shown on the Erosion Control Plan and/or Detail Sheet. 7) When the "existing" site/construction limits are completely stabilized, the contractor shall start striping the existing top soil within the construction limits and utilize in non-structural fill areas. 8) The contractor shall proceed to grade the remainder of the site.

9) Contractor shall excavate around existing manhole and expose the proposed connection point for the gravity sanitary sewer for this project. The contractor to keep all disturbances to the surrounding area to a minimum. Dirt, mud and debris shall be cleaned up and removed from the streets daily prior to the contractor leaving the site.

10) The contractor shall continue construction the proposed sanitary sewer. The trench area shall be seeded and mulched immediately following the installation of the sewer. 11) The proposed onsite storm sewer and sanitary sewer shall be installed concurrently with each other when crossings are encountered.

12) The contractor shall prepare the sub-grade for the proposed road system. If lime stabilization is the method chosen, dust shall be kept at a minimum. Dust shall be removed from the construction vehicles prior to leaving the site. 13) Contractor shall install all concrete curbing.

14) Contractor shall install water main. The trench area shall be seeded and mulched immediately following the installation of the water main. 15) Contractor shall have all other appropriate utilities installed. It is ultimately the contractor's responsibility to ensure that the trench area is seeded and mulched immediately following the installation of each utility.

16) Contractor shall install all asphalt pavement and concrete walk 17) The contractor shall permanent seed all areas disturbed by earth moving on the site 18) The contractor shall schedule a site inspection with the City/Town to ensure the site is stabilized. After the inspector approves the site conditions the

contractor shall remove all temporary erosion control practices. 19) The post-construction erosion control practices then become the responsibility of the developer of this project.

20) The developer of this project shall continue to monitor this site for good house keeping on the post-construction BMP's until a NOT is filed with IDEM.

CONTACT PERSON FOR EROSION CONTROL AND SEDIMENT PRACTICES

OWNER: STEVEN R. REILLY "HANCOCK LAND CO., LLC" 1111 W. MAIN ST., SUITE K GREENFIELD, IN 46140 (317) 462-7797

SEE DETAIL SHEETS FOR EROSION AND POLLUTION CONTROL DETAILS AS REFERENCED HEREON

SEEDBED PREPARATION:

1) Apply lime to raise the pH to the level needed for species being seeded. Apply 2-3 pounds of 12-12-12 analysis fertilizer (or equivalent) per 100 sq. ft. (approximately 1000 lbs, per acre) or fertilize according to test. Application of 150 lbs, of ammonium nitrate on areas low in organic matter and dertility will 2) Work the fertilizer and lime into the soil to a depth of 2-3 inches with a harrow, disk, or rake operated across the slope as much as possible.

3) Select a seed mixture based on projected use of the area (figure 5-2) while considering best seeding dates see figure 5-3. If tolerances are a problem, such as soil tolerances adjacent to streets and highways, see figure 5-4 before final selection.

Not more than 2

of the spill

hours after discovery

312 IAC 16-5-23 Spill reporting Authority: IC 14-37-3 Affected: IC 14-37

Section 23 (a) An owner or operator shall report all spills of oil or saltwater as required by Table 1 below:

SPILL REPORTING REQUIREMENTS:

More than 1000 gallons of oil or salt-

the boundary of the facility.

Less than 55 gallons of oil or salt-

water that is not contained in a secondary

IN Dept. of IN Dept, of Size and Location of the Spill Natural Resources Env. Management More than 2000 gallons of oil or salt-Not more than 48 hrs water that is contained in a secondary after discovery of spill containment structure approved by the Dept. of Natural Resources

containment structure approved by the Dept. of Natural Resources Less than 1000 gallons but more than 42 Not more than 48 hours

gallons of oil or saltwater that is not contained in a secondary containment structure approved by the Dept. of Natural Resources but is contained within

More than 55 gallons of oil that is not Not more than 2 contained within the boundary of the hours after discovery of the spill

Not more than 48 hours

water that is not contained within after the discovery of the boundary of a facility Any spill of oil or saltwater that

Not more than 2 enters waters of the State. hours after discovery of the spill Any spill of oil or saltwater: Not more than 2 (1) that has not been cleaned up hours after discovery in accordance with section 24 of the spill of this rule, or (2) for which cleanup has not been

started in accordance with Section 24 of this rule. Any spill of less than 42 gallons of

No report required. oil or saltwater that does not enter waters of the State.

(b) Spills required by Table 1 to be reported to the Indiana Department of Natural Resources must be reported to the Evansville Field Office by telephone at (812)-477-8773, or by facsimile at (812)-477-8952. (c) Spills required by Table 1 to be reported to the Indiana Department of Environmental Management must be reported to the Office of Environmental

Response at (317)-233-7745 or (888)-233-7745 (toll free in Indiana). (d) Each report of a spill must include all of the following information.

1. The name, address, and telephone number of the person making the report. 2. The name, address, and telephone number of a contact person, if different than the person making the report.

3. The location of the spill, including lease name, township, range and section. The time of the spill. 5. The identification of the substance spilled.

6. The approximate quantity of the substance that has been spilled or may be spilled. 7. The duration of the spill.

8. The source of the spill. 9. The name and location of waters damaged.

10. The identity of any response organization responding to the spill. 11. What measures have been or will be undertaken to perform a spill response. 12. Any other information that may be significant to the response action.

(Natural Resources Commission; 312 IAC 16-5-23; filed Sept. 11, 2000, 3:31 PM: 24: IR 279)

312 IAC 16-5-24 Spill Cleanup Authority: IC 14-37-3 Affected: IC 14-37

(a) An owner or operator shall clean up soils of oil, fluids contaminated with oil, or saltwater as required by this section. (b) Oil or fluid contaminated with oil that is confined within a secondary containment structure or collected as required by Section 22 of this rule must be:

1. remove within seventy-two (72) hours 2. placed in a non-leaking storage tank; and

3. managed or disposed of in accordance with Section 27(a) of this rule.

(c) Saltwater that is confined within a secondary containment structure or collected as required by Section 22 of this rule must be: 1 removed within seventy-two (72) hours,

2. placed in a non-leaking storage tank; and 3. disposed of in accordance with Section 27(b) of this rule.

(d) Fluid placed in a non-leaking storage tank under subsection (b) or subsection (c) must be disposed of in accordance with Section 27 or this rule with in thirty (30) days after discovery of the spill unless additional time is approved by the division.

(e) Soils contaminated with more than one (1) gallon of oil must be cleaned up as follows: 1. Soils that meet the conditions for remediation in Section 25(c) of this rule may be: a. remediate at the facility as required by Section 25 of this rule

b. applied to lease roads in accordance with Section 27(a)(1) of this rule; or c. excavated and disposed of as required by Section 27(d) of this rule. 2. Soils that do not meet the conditions for remediation in Section 25(c) of this rule must be excavated and disposed of as required by Section 27(e) of this

(f) Soils contaminated with saltwater must be cleaned up as required by Section 26 of this rule. (g) Soils contaminated with oil that will be remediate under Section 25 of this rule must be managed to prevent discharge of oil to unaffected soil or waters

(Natural Resources Commission; 312 IAC 16-5-24; filed Sept. 11, 2000, 3:31 PM: 24 IR 280)

EMERGENCY RESPONSE NUMBERS: -INDIANA DEPARTMENT OF NATURAL RESOURCES: 812-477-8773

-INDIANA DEPARTMENT OF ENVIRONMENTAL MANAGEMENT: 317-233-7745

MATERIAL HANDLING AND SPILL PREVENTION PLAN:

In order to minimize the release of potential pollutants during construction, the contractor shall implement this material handling and spill prevention plan. The contractor shall review this plan with all subcontractors and require that they implement the plan as well.

a) Fueling, lubrication and fluids: All operations involving the addition of fluids to equipment should be done in one location, as designated by the general contractor, or developer/owner, so that spills are limited to one location on the site, which will facilitate the cleanup of spills. If an onsite-fueling tank is planned to be on site, it shall be double walled and stored in this designed area. This location is an area that will not allow spilled fluids to migrate into subsurface soils. In the event of a spill, the fluid shall immediately be cleaned up by removing the contaminated soil or stone, which shall be disposed of in an acceptable manner. Spills on hard surfaces shall be soaked up by an acceptable material such as oil dry and the absorbent material disposed of in a proper manner. The spill shall also be reported immediately to the contractor's superintendent.

b) Equipment repair, especially when fluids must be removed from the equipment or the possibility of fluid spills is high, should always be done offsite at a facility that is more suitable than a construction site to handle spills. When equipment must be repaired onsite it shall be moved to the maintenance and fueling area if possible. Otherwise, suitable on site containers should be placed under the equipment during repair to catch any spilled fluids and these fluids should be disposed of in a proper manner.

c) All reusable fluid containers, such as gasoline cans, shall be inspected for leaks each time they are used. If leaks are found, the fluid shall be removed from the container in a proper manner and the container disposed of in an acceptable manner. Empty disposable container, such as grease tubes and lubricating oil and brake fluid containers, and their packaging, shall be disposed of in a proper manner and shall not be left on the ground or in the open on

Construction materials and their packaging:

a) Erosion control measures shown on the within plat shall be implemented prior to and during construction in the proper sequencing to minimize soil erosion. Erosion controls shall be inspected and maintained as described hereon. Excessive dusting of soil on the site shall be minimized by reducing construction traffic across bare soil during dry and/or windy weather, and by applying water or other acceptable dust control measures to the soil. Upon completion of construction and suitable establishment of permanent vegetation, temporary erosion control measures such as silt fence, check dams and inlet protection devices shall be removed in a manner to minimize additional land disturbance. Any areas disturbed by these operations shall be properly revegatated.

b) Large waste materials created by cutting, sawing, drilling, or other operations shall be properly disposed of in suitable waste containers. The site shall be checked at the end of the day, as a minimum, and all waste materials, including those blown across or off the site by wind shall be picked up and disposed of in suitable containers. Where possible, operations such as sawing that create small particles should be performed in one spot in an area protected from wind, and waste particles collected and disposed of frequently to minimize wind dispersal. Packaging used to transport materials to the site for construction shall be disposed of properly. Packaged materials stored onsite shall be inspected regularly and any loose packaging shall be repaired or disposed of

c) All dewatering of activities shall be done in accordance to good erosion control practices. These practices should include the use of dirt bags such as dandy dirt bags. The use of these types of dewatering devices will remove large quantities of silt, sediment, and dirt and prevent these materials to enter the storm sewer system.

d) If the use of lime is used to stabilize the soil of the site then all construction equipment used shall be cleaned of all excess material with water in the maintenance and refueling area as shown within these plans.

e) Nutrients and fertilizers shall only be used to establish rapid vegetation. When these products are utilized, the user should pay strict attention to the products recommended usage.

a) The washout shall be inspected daily and after each strom event and repaired as needed.

b) Excess concrete should be removed when the washout system reaches 50 percent of the design capacity and repaired as needed. c) Dispose of or recycle all concrete in a legal manner. d) Concrete washout systems are designed to promote evaporation. However, if the liquids do not evaporate and the system is near capacity it may be

necessary to vacuum or remove the liquids and dispose of them in an acceptable method. e) Inspect construction activities on a regular basis to ensure suppliers, contractors, and other are utilizing designated washout areas. If concrete waste is being disposed of improperly, identify the violators and take appropriate action. f) When concrete washout systems are no longer required, the concrete washout system shall be closed. Dispose of all hardened concrete and other

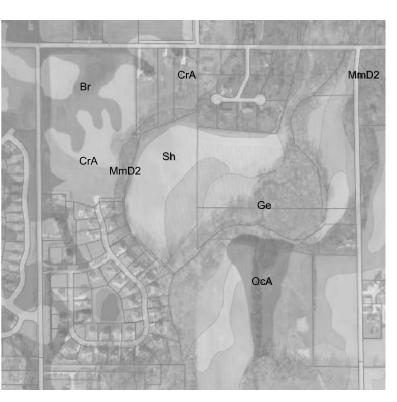
materials used to construct the system. g) Holes, depressions and other land disturbances associated with the system should be backfilled, grades, and stabilized.

4. Paint products:

Concrete waste water:

a) All excess paint and their related products shall be disposed of in the manner at which the manufacturer suggests. Under no circumstances will paint or their related products be cleaned or disposed of in soil, sanitary sewers, storm sewers or detention basins. Any violation of this shall be reported to the job

In the event of accidental contamination, all efforts should be made to remove contaminants in an appropriate manner. The appropriate authorities should be contacted immediately to determine if further measures are needed.



SOILS MAP:

CrA (Crosby silt loam)

MmD2 (Miami silt loam)

0 to 3 percent slopes. This nearly level soil is in slightly convex areas on uplands. Areas are

Br (Brookston silty clay loam)

O to 2 percent slopes. This nearly level soil is in depressions, swales, and narrow drainageways on uplands. Areas are irregular in shape and 2 to 400 acres in size. Because of continuous farming in some areas, the dark-colored surface layer is eroded and only 8 inches of it remains. Slope is dominantly less than 1 percent. Runoff is very slow or ponded.

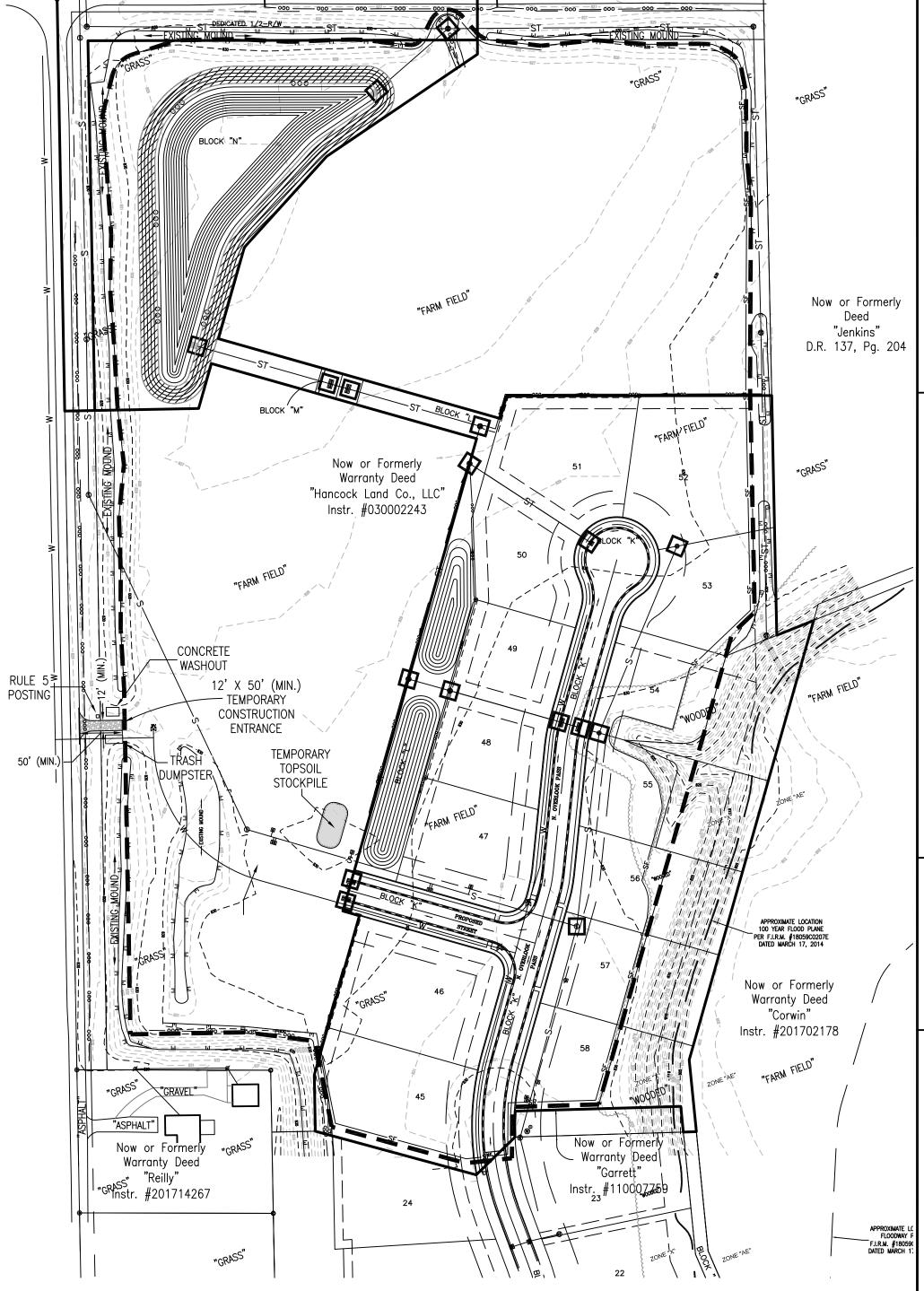
SOILS DESCRIPTION

irregular in shape and 2 to 250 acres in size. Slope is dominantly 1 to 3 percent. Runoff is slow.

12 to 18 percent slopes. Eroded. This soil is along breaks between the uplands and the bottom

lands. Areas are long, narrow, and winding in shape and 2 to 40 acres in size. The soil has a profile similar to the one described as representative of the series, but the combined thickness of

the surface layer and subsoil is dominantly less than 26 inches. Runoff is rapid.



EROSION LEGEND:

- CONSTRUCTION LIMITS — INLET PROTECTION - EROSION CONTROL BLANKET

LEGEND:

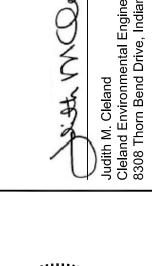
NOTE: ALL DISTURBED AREAS TO BE PERMANENTLY SEEDED IMMEDIATELY AFTER CONSTRUCTION OF SITE.

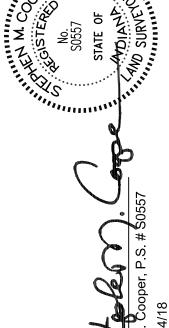
Assumed North

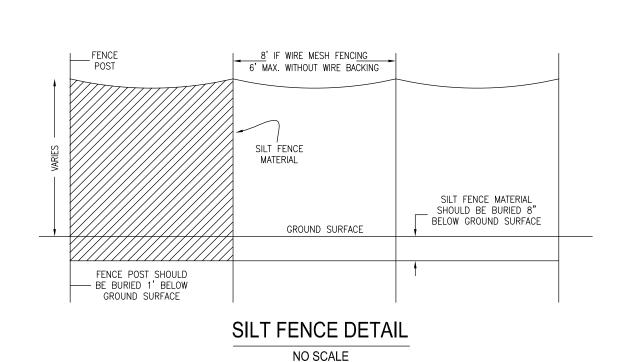
OHE - OVERHEAD UTILITY LINE ——————— — SANITARY SEWER LINE ——ST———— — STORM SEWER LINE ——————— — UNDERGROUND WATER MAIN - APPROX. EDGE OF WOODS

(D) - DEED DIMENSION

- (M) MEASURED DIMENSION (P) - PLAT DIMENSION \varnothing - EXISTING UTILITY POLE
- WY EXISTING WATER VALVE W - EXISTING WATER METER ⊙ − EXISTING WATER LATERAL
- O REBAR/PIPE FOUND - INDICATES 1/4 SECTION (ST) - STORM SEWER MANHOLE
- S SANITARY SEWER MANHOLE €3 - STORM SEWER BEEHIVE # - STORM SEWER INLET U - CURB INLET
- S SANITARY LATERAL BENCHMARK 999.99 - SPOT ELEVATION
- ₩ EXISTING FIRE HYDRANT 999 – EXISTING CONTOUR

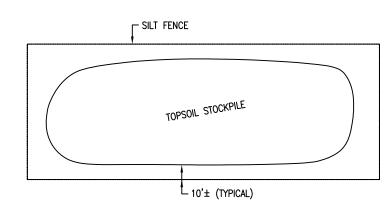




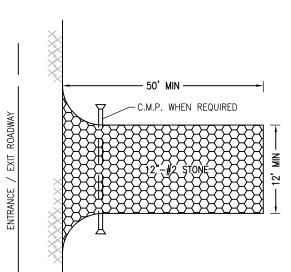


POST 2"X2" WOOD OR 1.33 LBS/FOOT STEEL 5' MAX. SPACING NON-WOVEN FILTER FABRIC - SECURE TO POSTS WITH 2" LATH 4"X4" TRENCH TO BE BACKFILLED AND COMPACTED INSPECT WEEKLY AND AFTER EACH STORM EVENT. 2. IF FABRIC IS TORN OR DETERIORATED, REPLACE SECTION IMMEDIATELY 3. REMOVE SEDIMENT WHEN HEIGHT REACHES 1/3 OF THE HEIGHT OF THE FENCE OR IS CAUSING FABRIC TO BULGE. 4. TAKE CARE NOT TO UNDERMINE THE ENTRENCHED FABRIC. 5. AFTER DRAINAGE AREA IS STABILIZED, REMOVE FENCE AND SEDIMENT, BRING DISTURBED AREA TO GRADE AND STABILIZE IT.

SILT FENCE SECTION (NO SCALE)



TYPICAL TOPSOIL STOCKPILE NO SCALE



1.) FILTER FABRIC TO BE USED UNDER STONE. 2.) INSPECT ENTRANCE PAD WEEKLY AND AFTER

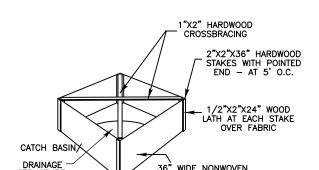
ADEQUATE DRAINAGE AND RUNOFF CONTROL. IMMEDIATELY REMOVE MUD AND SEDIMENT FROM

PUBLIC ROADS BY BRUSHING OR SWEEPING. IF FLUSHING IS REQUIRED, USE SEDIMENT TRAP OR

6.) REPAIR BROKEN PUBLIC ROADWAYS IMMEDIATELY.

TEMPORARY GRAVEL CONSTRUCTION ENTRANCE / EXIT PAD DETAIL

NO SCALE

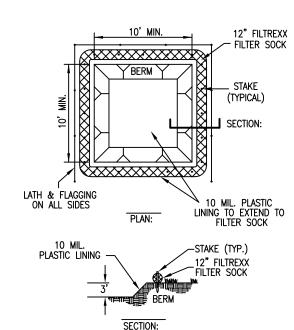


 INSPECT WEEKLY AND AFTER EACH STORM EVENT. 1.) INSPECT WEEKLY AND AFTER EACH STORM EVENT IF FABRIC IS TORN OR DETERIORATED, REPLACE SECTION IMMEDIATELY. 2.) REMOVE SEDIMENT WHEN HEIGHT REACHES 1/3 OF THE HEIGHT OF THE FENCE OR IS CAUSING FABRIC TO BULGE.
3.) TAKE CARE NOT TO UNDERMINE THE ENTRENCHED FABRIC.
4.) AFTER DRAINAGE AREA IS STABILIZED, REMOVE FENCE AND SEDIMENT, BRINGING DISTURBED AREA TO GRADE AND STABILIZE IT.
5.) REFER TO SILT FENCE SECTION FOR EMBEDMENT DETAIL.

SILT FENCE INLET PROTECTION DETAIL: (NO SCALE)

Species	Seedir	ng Rate	Suitable pH	Site S	uitability	
	lbs/acre	lbs/1000 sq. ft.		Droughty	well Drained	Wet
Level & Sloping (open ar	eas)					
1.) Tall Fescue	35	0.8	5.5-8.3	2	1	2
2.) Tall Fescue Red Clover**	25 5	0.6 0.12	5.5-8.3	-	1	-
3.) Kentucky Bluegrass Creeping Red Fescue	15 15	0.4 0.4	5.5-7.5	2	1	-
Steep Banks & Cuts		1		•		•
4.) Tall Fescue Kentucky Bluegrass	15 25	0.4 0.6	5.8-7.5	2	1	2
5.) Tall Fescue Emerald Crownvetch**	35 10	0.8 0.25	5.5-8.3	2	1	-
Lawns & High Maintenar	ice Areas			,		
6.) Kentucky Bluegrass Creeping Red Fescue	40 40	0.9 0.9	5.8-7.5	2	1	-
7.) Perennial Ryegrass (Turf Type)	170	4.0	5.0-7.5	-	1	-
8.) Tall Fescue	170	4.0	5.5-8.3	2	1	2

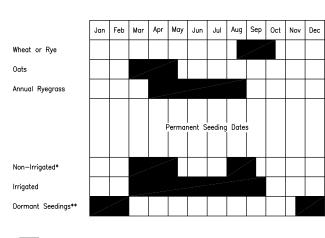
* 1-Preferred 2-Will Tolerate ** Innoculate with specific innoculants Figure 5-2: Permanent Seed Mixtures



1. WASHOUT LOCATION SHOWN ON EROSION CONTROL PLAN. 2. DRY CONCRETE TO BE REMOVED FROM SITE PERIODICALLY AND DISPOSED OF BY APPROVED METHODS. 3. THE CONCRETE WASHOUT SIGN SHALL BE INSTALLED

WITHIN 30' OF THE TEMP. CONCRETE WASHOUT FACILITY.

"BELOW GRADE" **CONCRETE WASHOUT** NO SCALE

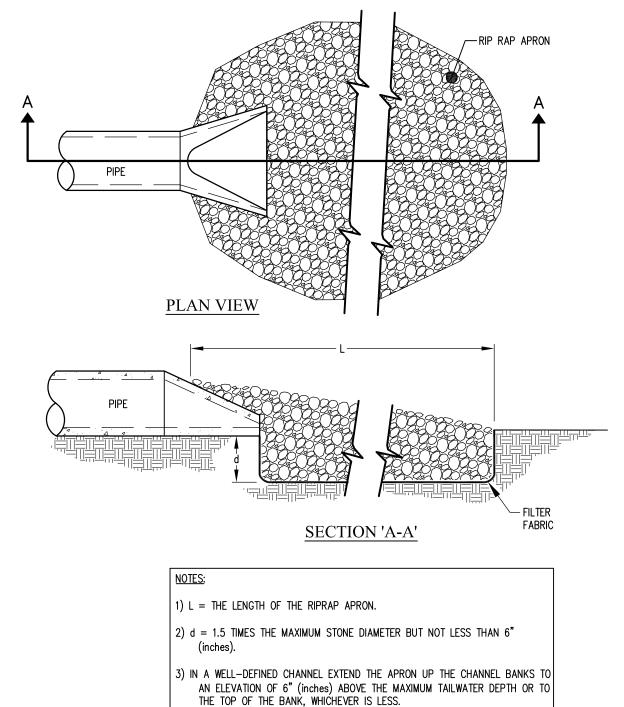


Irrigation needed during this period. To control erosion at times other than in the shaded areas, use mulch. * Late summer seeding dates may be extended 5 days if mulch is applied. ** Increase seeding application by 50%.

Figure 5-3: Temporary Seeding Dates

	С	Soil onditio	on	Shade Tolerance	Close Mowing 2 - 3 1/2 ln.	Tramping Tolerance	Fertility Needs	Winter Hardness	Flooding Tolerance Days	Mature Height Inches	Emmergence Time Days	To	Salt oleran	ce
	Wet	Norm	Dry		2 2 2				Tole	Hei	뜹	Gen	Soil	Spray
Creeping Red Fescue Festuca rubra	2	1	2	1	1	1	Med	1	20-25	12-18	7-21			s
Kentucky Bluegrass Poa pratinals	2	1	2	1	1	1	Med	1	25-35	12-18	10-20			мт
Tall Fescue Festuca L arundinacea	2	1	1	1	1	1	Low	1	24-35	24-36	5-14		Т	
Perennial ryegrass Lollum perenne	2	1	2	-	1	2	Med High	2	15-20	12-18	5-10		мт	
Crownvetch Coronilla varia	-	1	1	2	-	-	Low	1	5-10	24	14-21	т		
Red Clover Trifolium pratense	-	1	-	2	-	-	Med	1	7–10	18	5-10	s	S	
			1				T – MT -	Toler - Med		erance	soil salts	& sp	oray):	

Figure 5-4

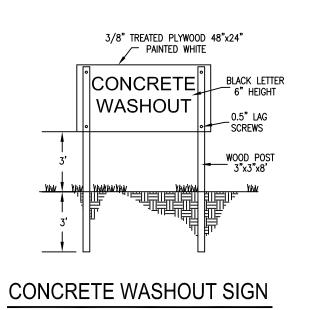


RIPRAP INSTALLATION

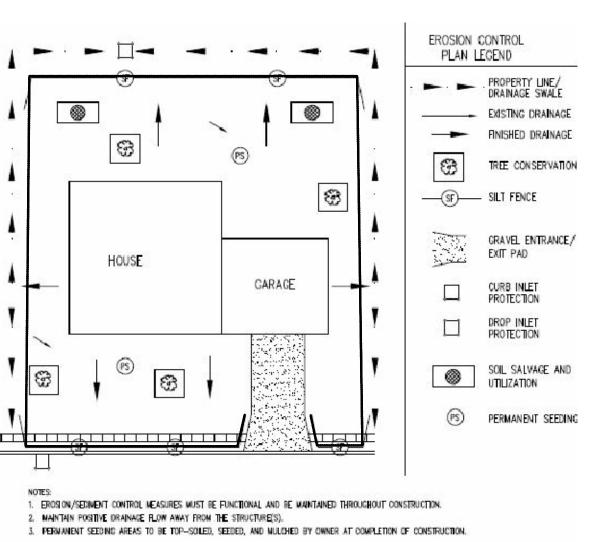
NO SCALE

RIPRAP AND SOIL FOUNDATION.

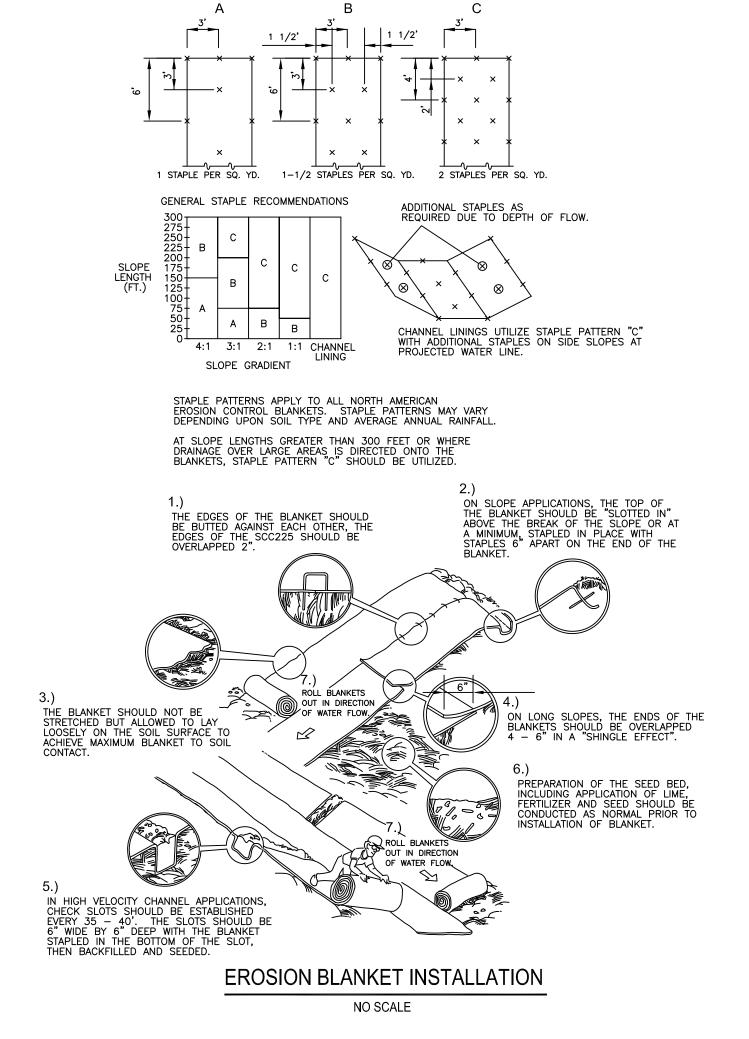
4) A FILTER BLANKET OR FILTER FABRIC SHOULD BE INSTALLED BETWEEN THE

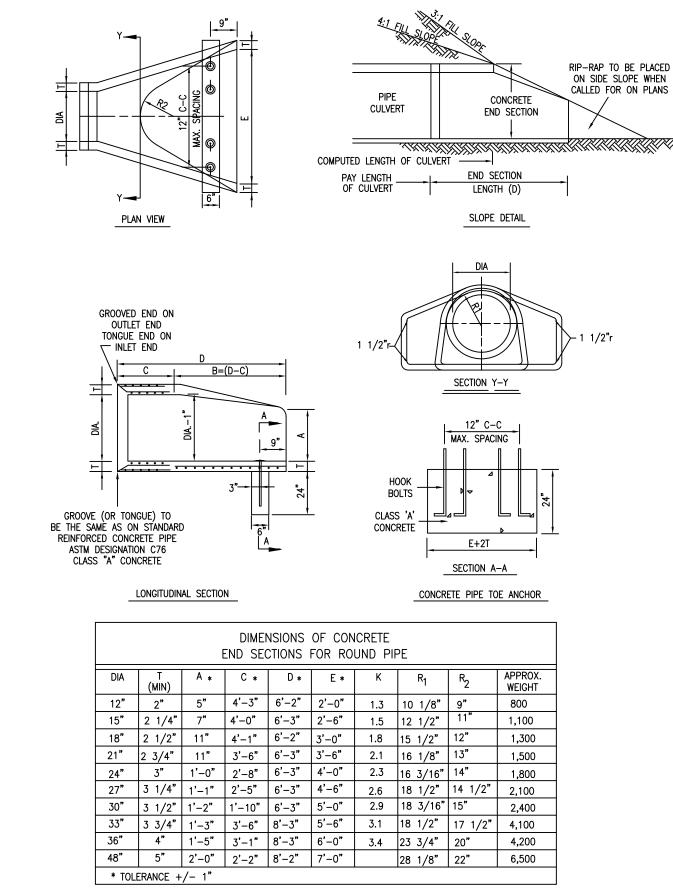


NO SCALE

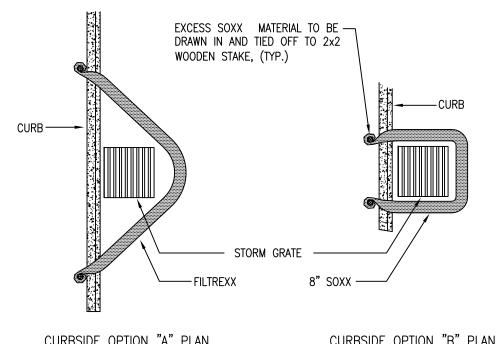


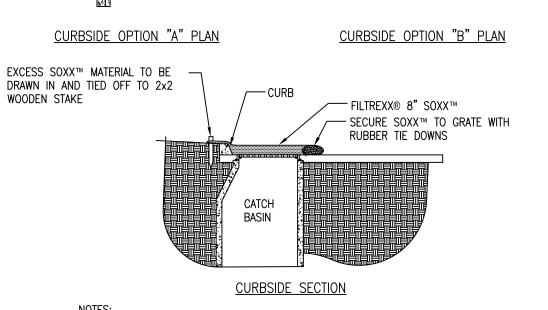
INDIVIDUAL LOT STORM WATER POLLUTION & PREVENTION DETAIL NO SCALE











NOTES:

1. ALL MATERIAL TO MEET FILTREXX SPECIFICATIONS. FILTER MEDIA FILL TO MEET APPLICATION REQUIREMENTS. COMPOST MATERIAL TO BE DISPERSED ON SITE, AS DETERMINED BY ENGINEER. FILTREXX INLET PROTECTION

NO SCALE

EXCESS SOCK MATERIAL TO BE DRAWN IN AND TIED OFF TO STAKE AT BOTH ENDS FILTREXX? CHECK DAM-SIZED TO SUIT CONDITIONS 8" TO 18" TYPICAL ← FLOW 2" X 2" X 36" WOODEN -STAKES PLACED 5' O.C.

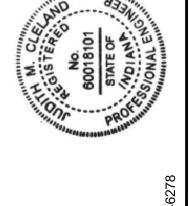
NOTES:

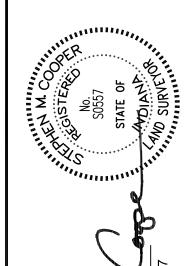
1. ALL MATERIAL TO MEET FILTREXX? SPECIFICATIONS. . CHECK DAM SHOULD BE USED IN AREAS THAT DRAIN 10 ACRES OR LESS. SEDIMENT SHOULD BE REMOVED FROM BEHIND CHECK DAM ONCE THE ACCUMULATED HEIGHT HAS REACHED ½ THE HEIGHT OF THE CHECK DAM.
4. CHECK DAM CAN BE DIRECT SEEDED AT THE TIME OF INSTALLATION. 5. CONTRACTOR IS REQUIRED TO BE A FILTREXX CERTIFIED? INSTALLER.

FILTREXX CHECK DAM NO SCALE

SPECIFICATIONS AND DETAILS NOTE

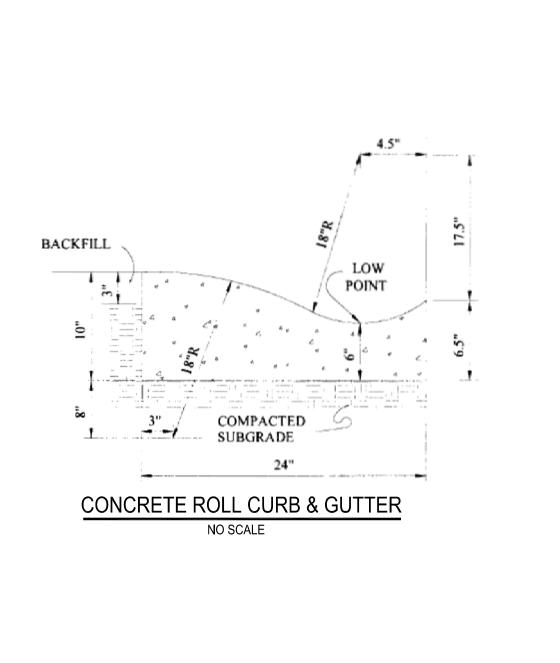
- TOWN OF CUMBERLAND DETAILS AND SPECIFICATIONS SUPERCEDE ANY CONFLICTING INFORMATION BETWEEN THE GENERAL DETAILS (SHEETS C10.0 - C10.1) AND THE TOWN OF CUMBERLAND (SHEETS C10.2 - C10.4).

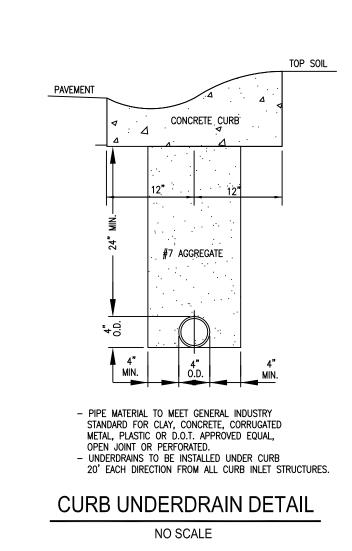


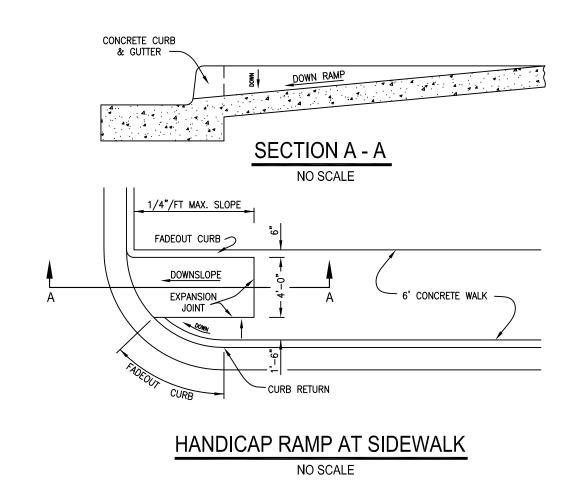


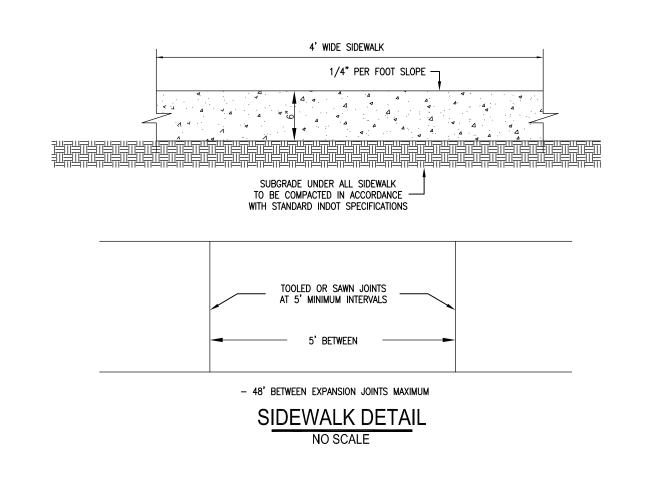
Sheet Number

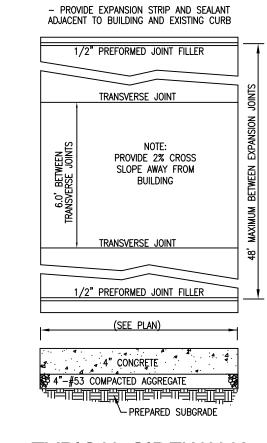
C10.0



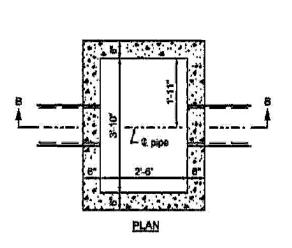


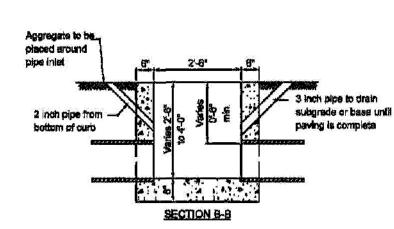




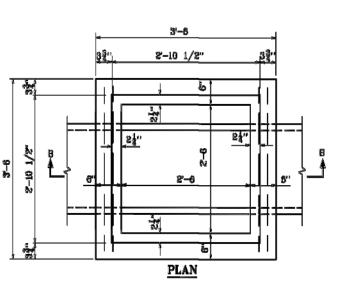


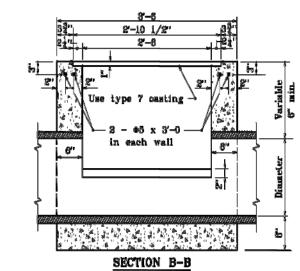
TYPICAL SIDEWALK **SECTION & JOINT DETAIL** NO SCALE



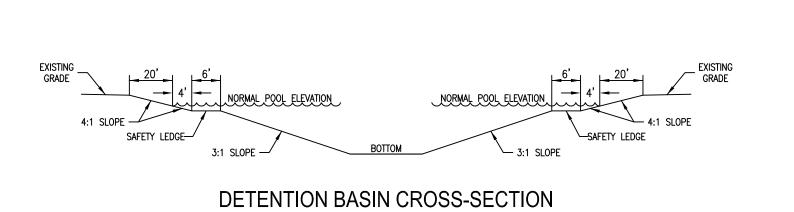


INLET-TYPE C TYPE "C" CONCRETE INLET (REFERENCE INDOT STANDARD DRAWING # E 720-INST-02) NO SCALE

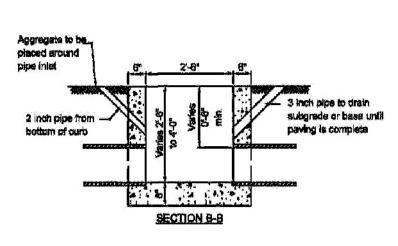


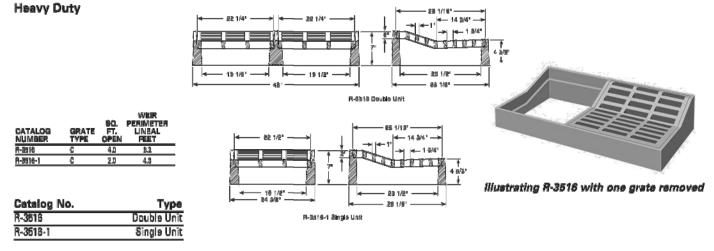


TYPE "F" CONCRETE INLET (REFERENCE INDOT STANDARD DRAWING # E 720-INST-04) NO SCALE



NO SCALE



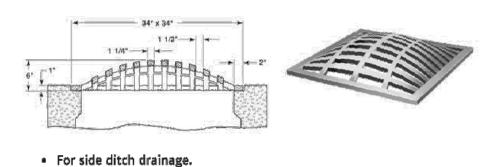


SINGLE/DOUBLE ROLL CURB INLET (NEENAH CASTING NO. R-3516 OR R-3516-1 OR APPROVED EQUAL) NO SCALE

R-4215-C

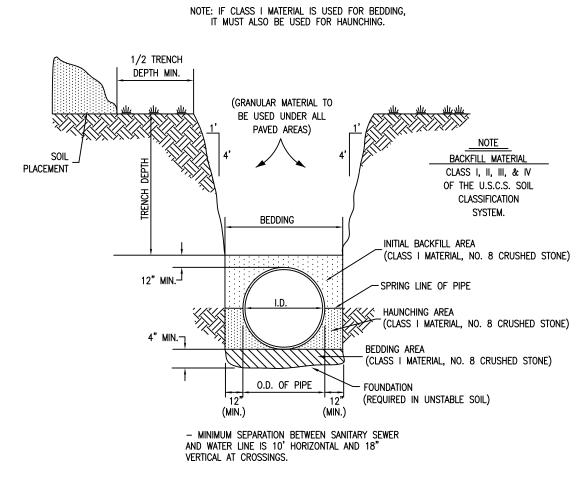
Convex Grate

R-3516 Series
Inlet Frame, Grate for Driveway and Mountable Curb



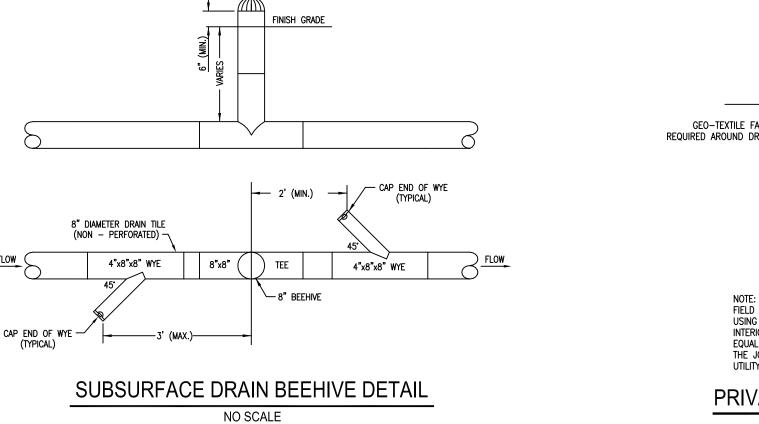
R-A215-C Beehive 3.3 11.3	Catalog Number	Grate Type	Sq. Feet Open	Weir Perimeter Lineal Feet
1, 421,7	R-4215-C	Beehive	3.3	11.3

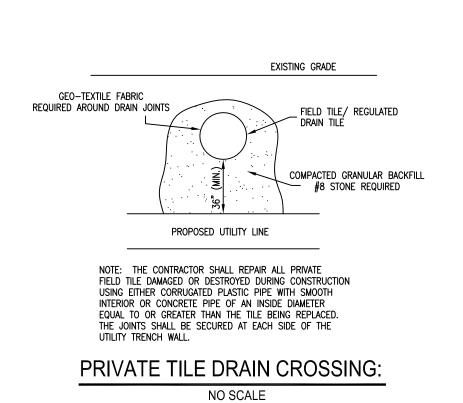
BEEHIVE INLET CASTING DETAIL (NEENAH CASTING NO. R-4215-C OR APPROVED EQUAL) NO SCALE

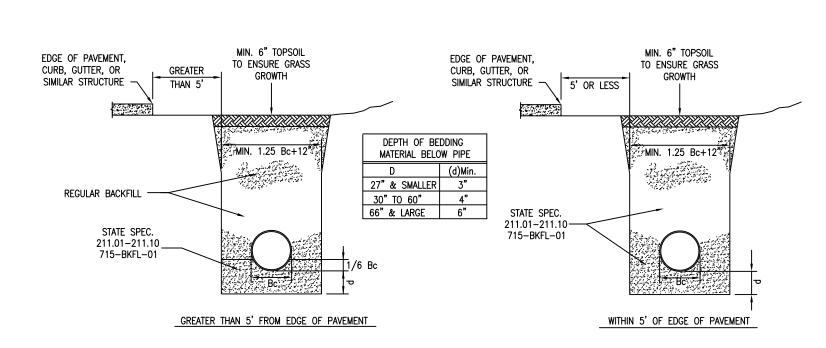


REQUIRED COMPACTION TABLE % STANDARD PROCTOR DENSITY (ASTM D 2321)

TRENCH & PIPE EMBEDMENT DETAIL NO SCALE



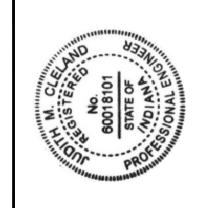


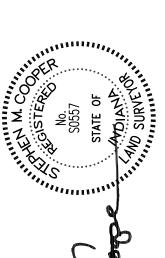


REINFORCED CONCRETE PIPE (RCP) BEDDING DETAIL NO SCALE

SPECIFICATIONS AND DETAILS NOTE

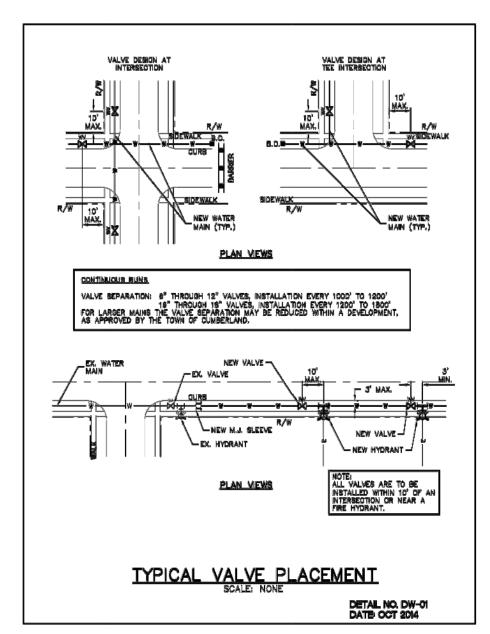
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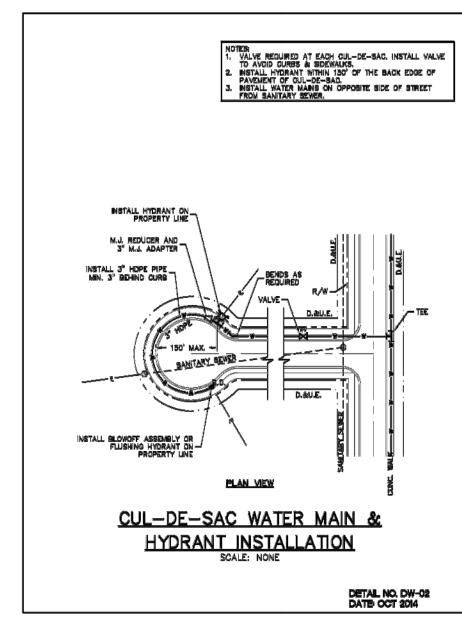


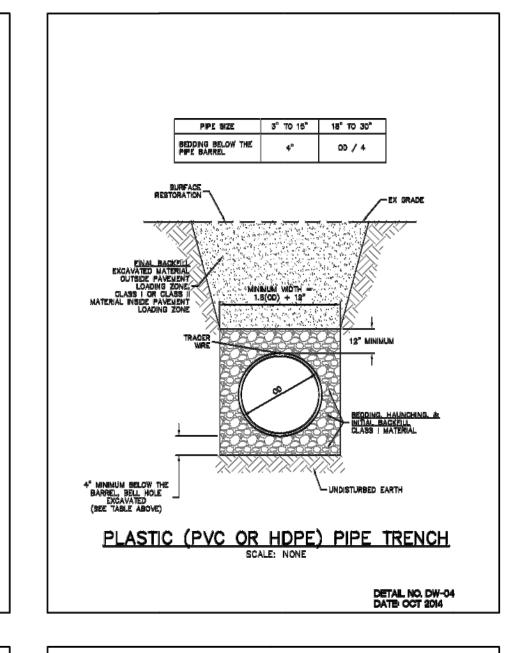


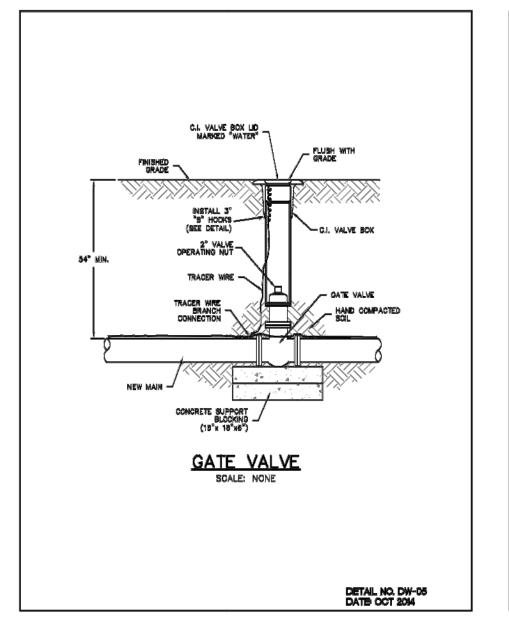
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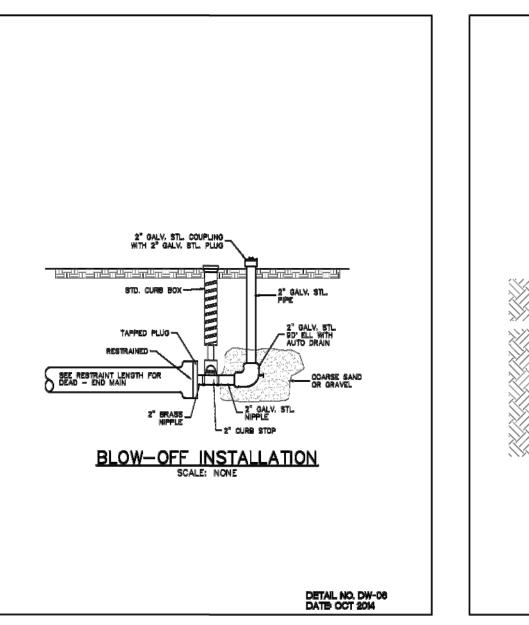
C10.1

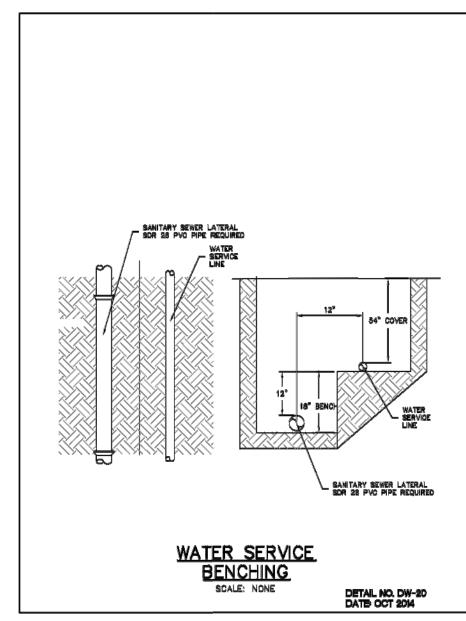


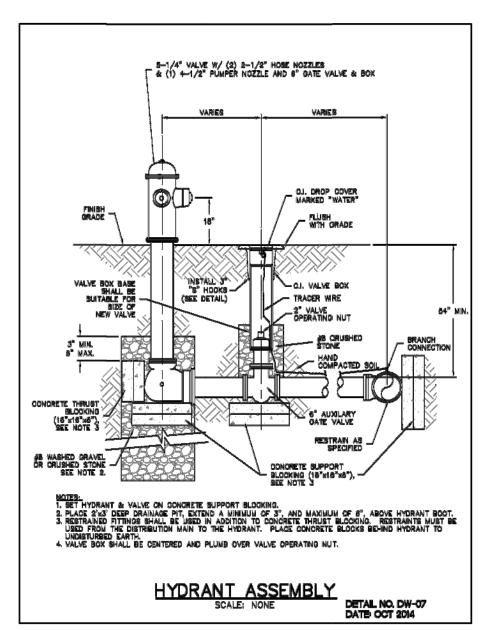


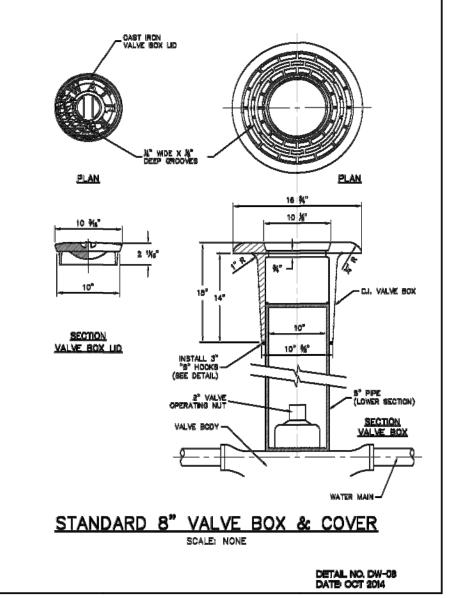


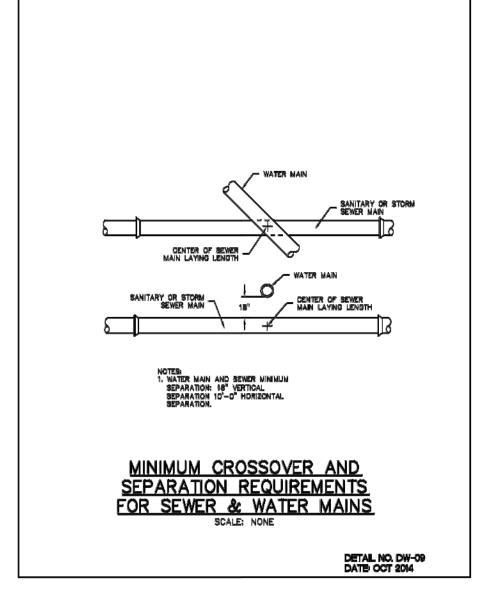


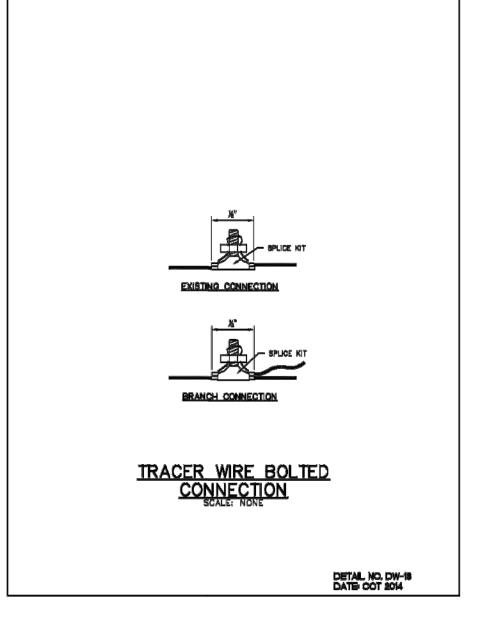


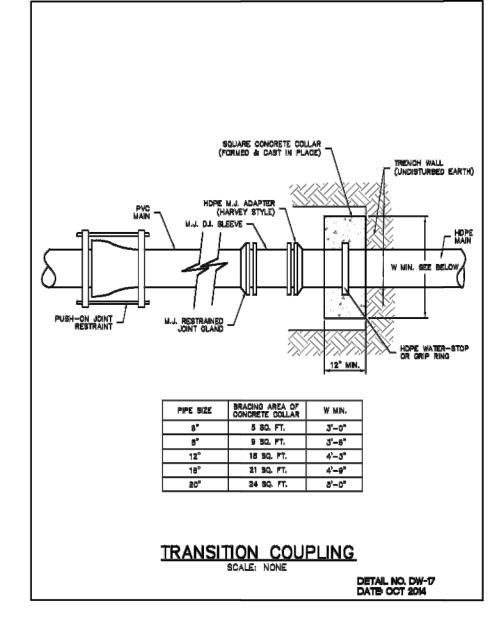


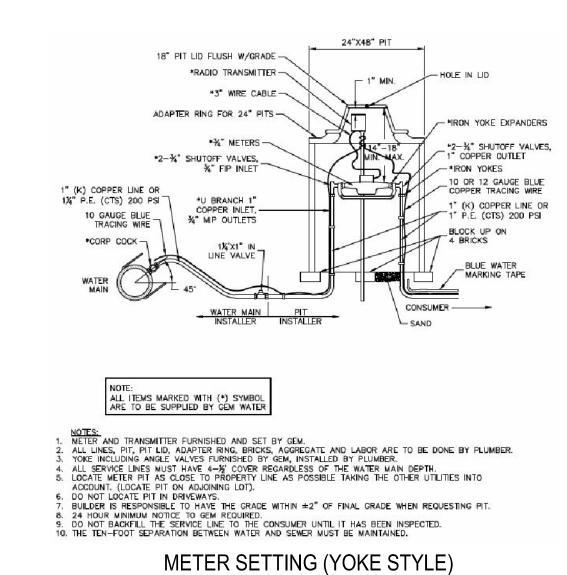




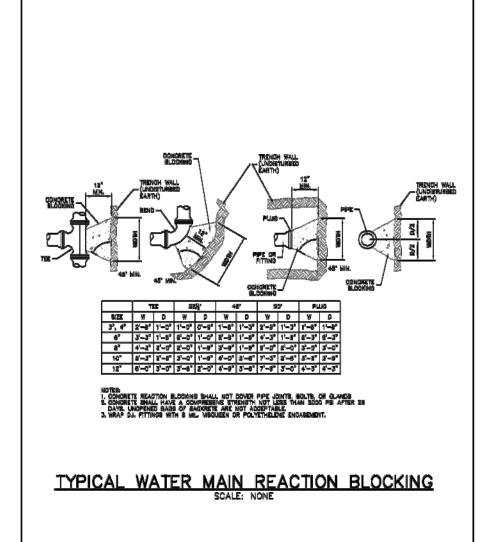




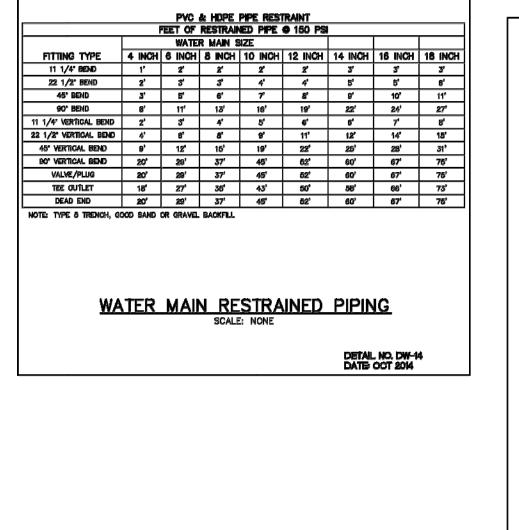


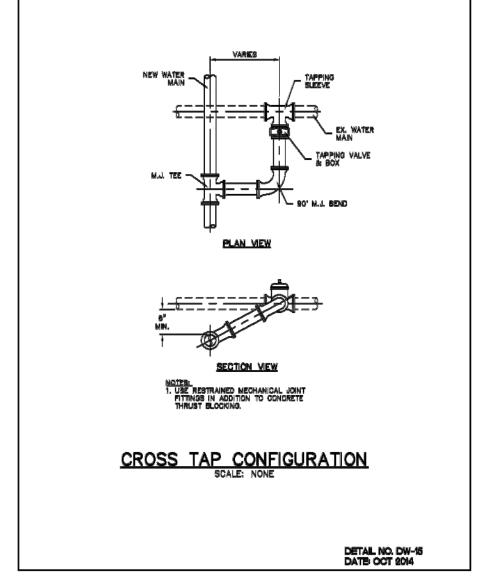


NO SCALE



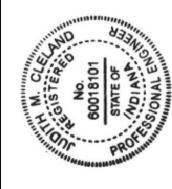
DETAIL NO. DW-13 DATE: OCT 2014





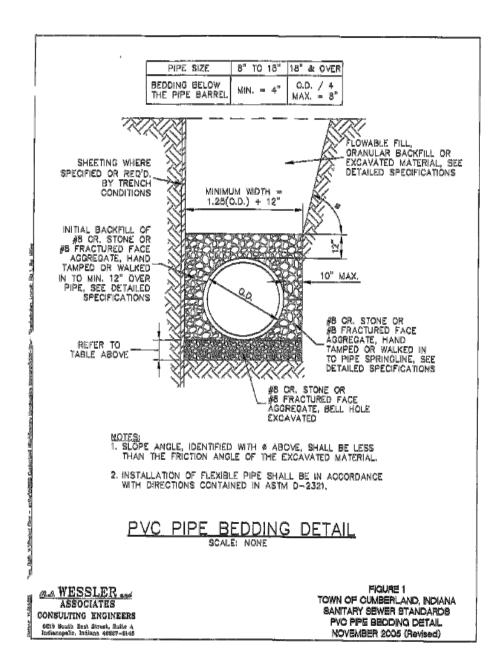


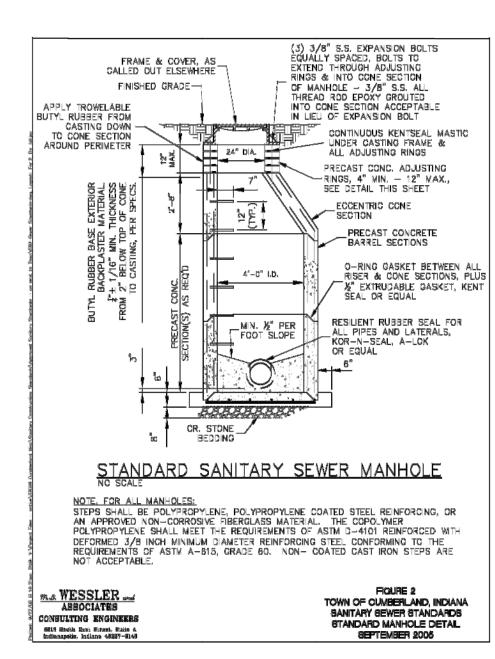
- TOWN OF CUMBERLAND DETAILS AND SPECIFICATIONS SUPERCEDE ANY CONFLICTING INFORMATION BETWEEN THE GENERAL DETAILS (SHEETS C10.0 - C10.1) AND THE TOWN OF CUMBERLAND (SHEETS C10.2 - C10.4).

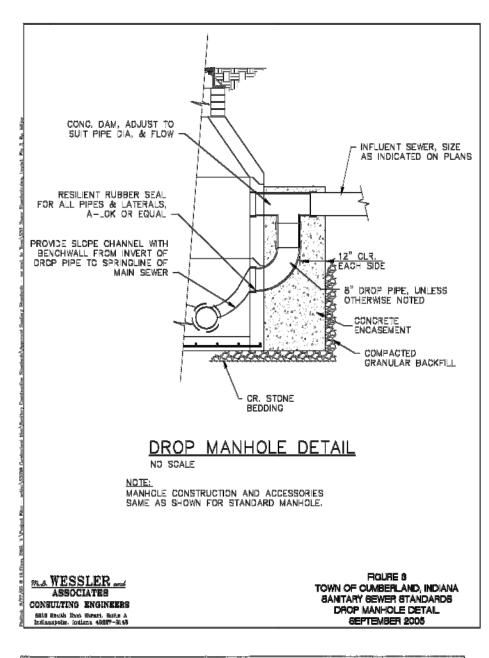


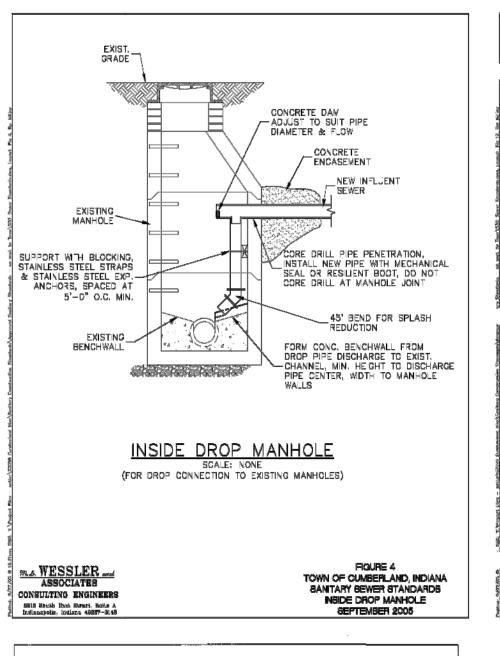
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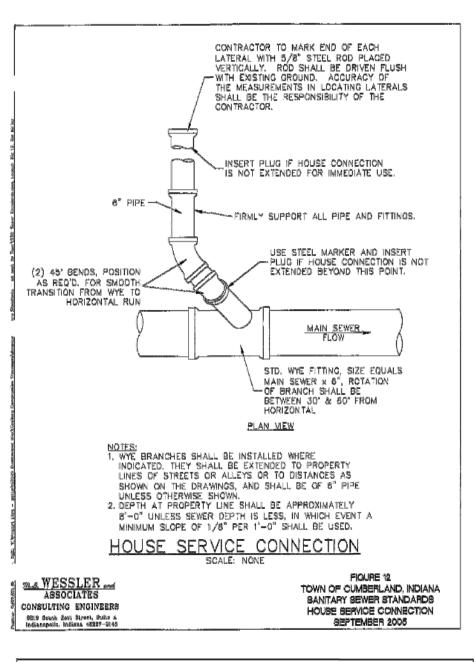
C10.2











NEW SDR 35 REPAIR PIPE SECTION SIZE TO MATCH EXISTING

INFORMATION SHOWN MAY BE MODIFIED SUBJECT TO THE REQUIREMENTS OF HANCOCK

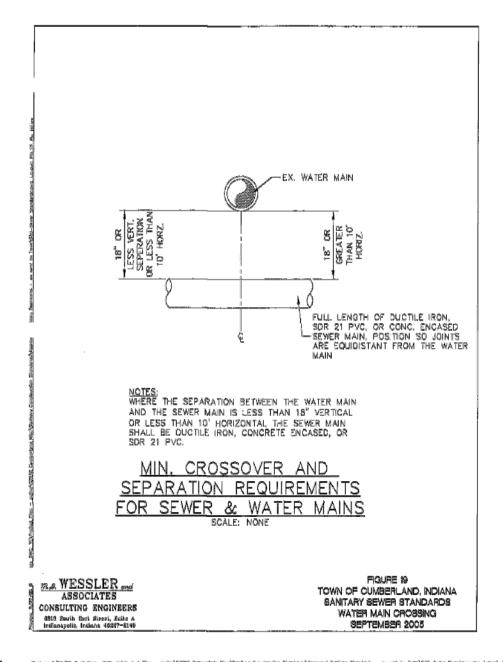
COUNTY OR MARION COUNTY

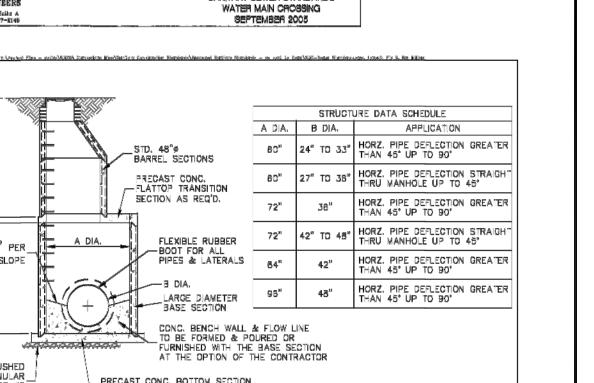
FIGURE 20 TOWN OF CUMBERLAND, INDIANA BANITARY BEWER STANDARDS FIELD TILE REPAIR DETAIL

FLEXIBLE TRANSITION COUPLING

-ATTACHED WITH STAINLESS STEEL BANDS

FIELD TILE REPAIR DETAIL

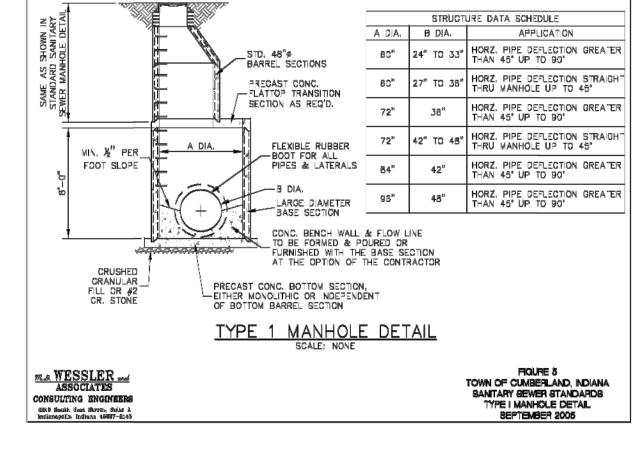




MIN. STEEL AREA 0.12 SQ. IN./LFT, OF WIDTH IN BOTH DIRECTIONS

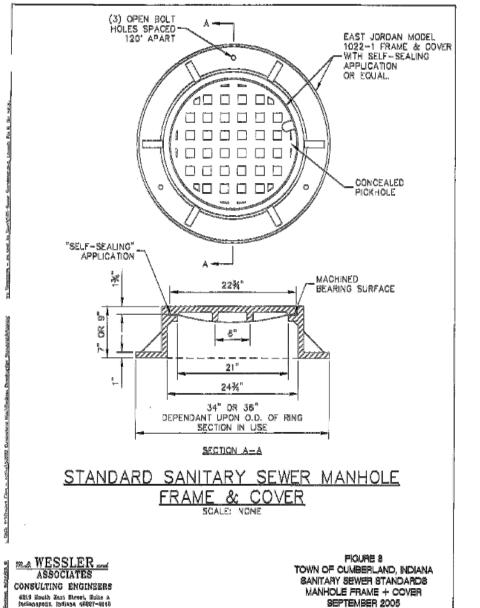
OPENING ADDITIONALLY REINFORCED WITH EQUIVALENT OF 0.20 SQ. IN./LFT. © 9D STRAIGHT RODS, MINIMUM LENGTH = DIA. OF OPENING PLUS 2 IN.

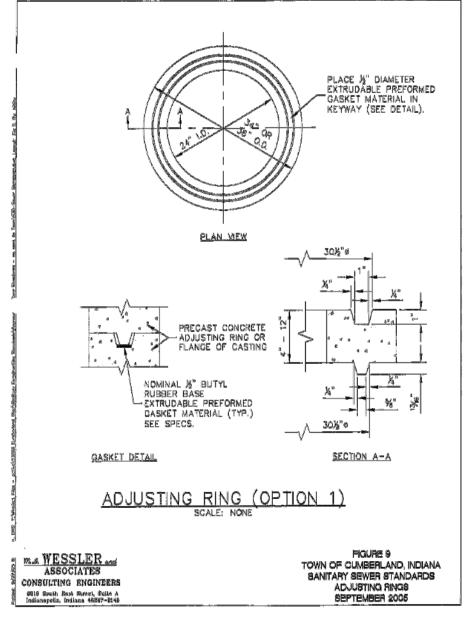
TOWN OF CUMBERLAND, INDIANA SANITARY SEWER STANDARDS MANHOLE TRANSITION DETAIL

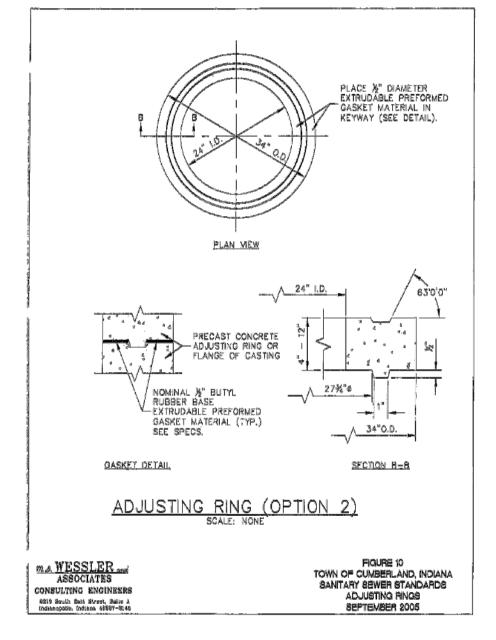


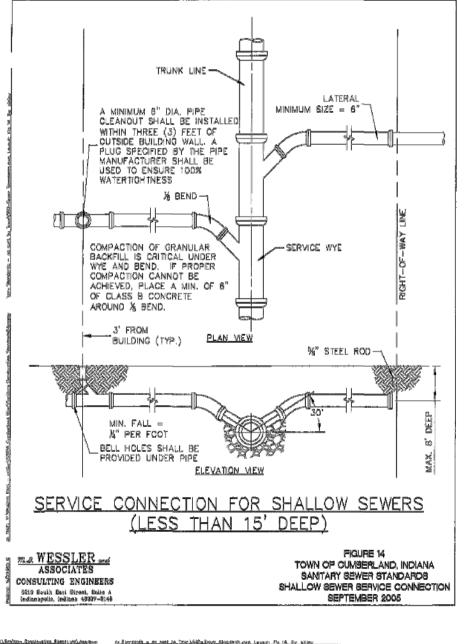
REDUCER CAP FOR PRE-CAST

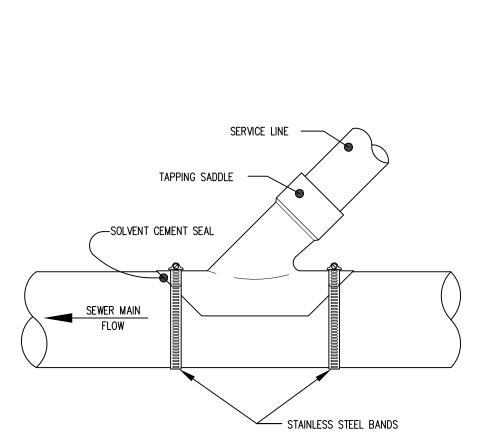
MANHOLE SECTIONS
SCALE: NONE









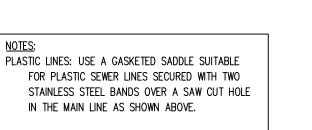


EXISTING FIELD TILE __ SIZE AND MAT_, VARIES

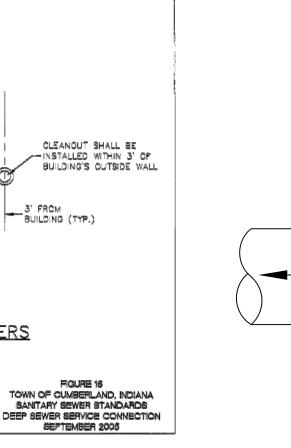
MESSLER ____ ASSOCIATES

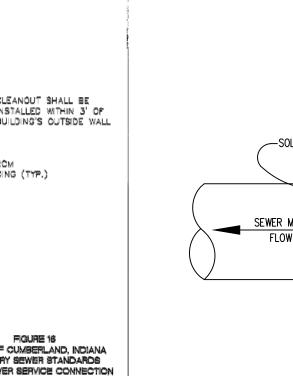
CONSULTING ENGINEERS

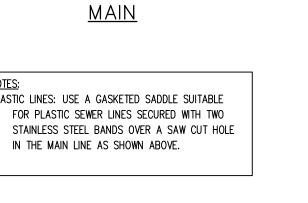
SELS Evelh That Street, Seits A Indianapolis, Indiana 49887—3148

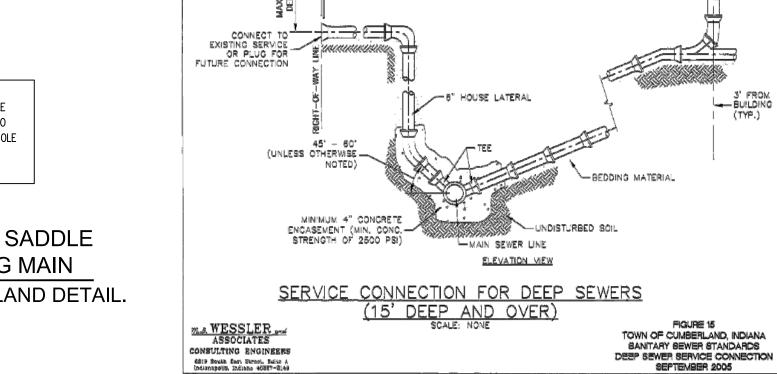


SANITARY SEWER LATERAL SADDLE CONNECTION TO EXISTING MAIN NOTE: NOT A TOWN OF CUMBERLAND DETAIL.







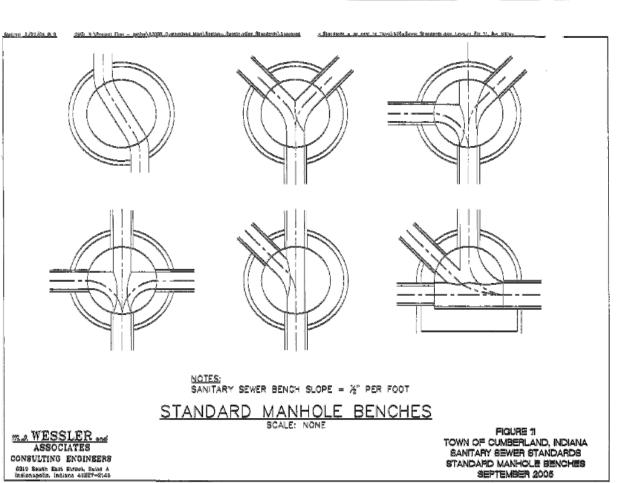


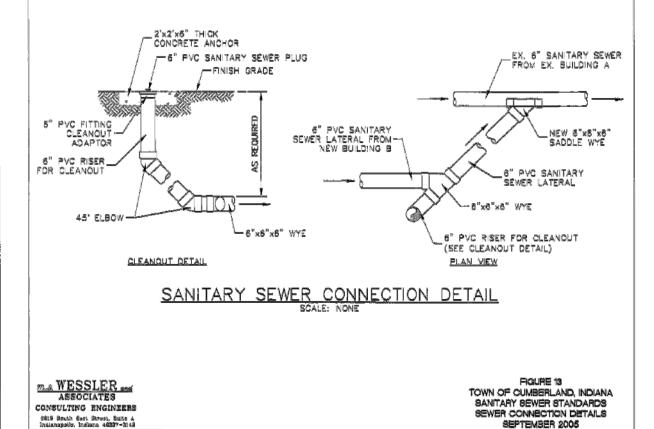
√-%" STEEL ROD

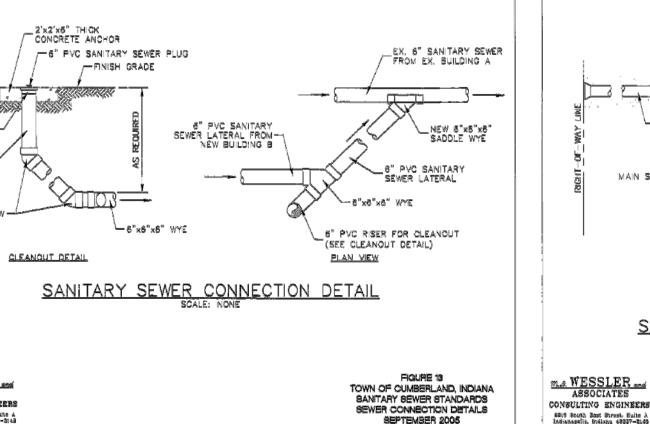
SECTION D & H

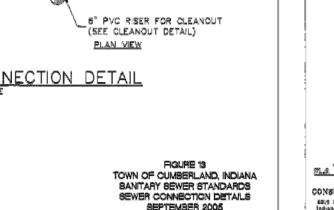
80" 8'-0" 72" 7'-2"

CONSULTING ENGINEERS 6519 South East Street, Saite A Indianapais, intlana 45687-0146









SADDLE CONNECTIONS SHALL BE AS FOLLOWS:

NOTES: 1. NORMAL LATERAL SLOPE IS M" PER FOOT MINIMUM LATERAL SLOPE IS M" PER FOOT 2. MINIMUM COVER IS 4 FEET OVER PIPE

SERVICE CONNECTION FOR DEEP SEWERS

(15' DEEP AND OVER)

- 1. THE CONNECTIONS SHALL BE A SEATED SADDLE THAT WILL NOT ALLOW THE LATERAL TO PROTRUDE INTO THE MAINLINE SEWER.
- 2. THE SADDLE SHALL HAVE AT LEAST TWO (2) STAINLESS STEEL BANDS CONNECTING AROUND THE EXISTING SEWER.
- 3. THE CUT INTO THE EXISTING SEWER SHALL BE CORE DRILLED. SAW CUTS AND HAMMER TAPS ARE NOT ALLOWED.
- 4. DEVELOPER TO INSTALL A CONCRETE CRADLE AROUND EACH LATERAL SADDLE CONNECTION TO DISCOURAGE ANY FUTURE PIPE SETTLEMENT



Sheet Number

C10.3

SPECIFICATIONS AND DETAILS NOTE

- TOWN OF CUMBERLAND DETAILS AND SPECIFICATIONS SUPERCEDE ANY CONFLICTING INFORMATION BETWEEN THE GENERAL DETAILS (SHEETS C10.0 - C10.1) AND THE TOWN OF CUMBERLAND (SHEETS C10.2 - C10.4).

Any sewer, gas, water, or other pipes or conduits crossing the trench shall be supported without damage and without interrupting service. The manner of supporting such pipes or conduits shall be subject to the approval of the Town and/or the inspector of the utility involved.

The Contractor shall provide adequate sheeting and bracing in open cut trenches to protect life, property and the work.

All sheeting, planking, timbering, shoring, bracing, and bridging shall be placed, renewed and maintained, and shall not be removed until sufficient backfill has been placed to protect the pipe.

Where rock is encountered during trenching operations, the Contractor may remove the rock by mechanical means. The use of a rock trencher which produces excavated material commensurate to "granular backfill" is preferred. Materials suitable for granular backfill excavated by a rock trencher may be used as bedding for pipe in areas of rock excavation. If the Contractor removes the rock by the use of controlled explosives, he shall employ a licensed blasting supervisor and crews experienced in explosive demolition and excavation. Prior to any blasting operations, the Owner/Contractor shall submit to the Town the blasting supervisor's experience record, a monitoring and control program showing the blasting area, charge per round in pounds, and storage procedures for explosives. No farm fences shall be cut when gates are available within a reasonable distance to move equipment from one field to another.

8.03 Bedding and Backfill A. General

All trenches or excavations shall be backfilled to the original surface of the ground or such other grades as required or directed. In general the backfilling shall be carried along as speedily as possible in order to avoid open excavations.

B. Bedding and Backfill Materials

Bedding and Backfill material classes referenced within this section shall be defined as follows:

Class I Angular, 6- to 40-millimeters (1/4- to 11/2-inch) graded stone such as crushed stone. A No. 8 gravel possessing a minimum 50% mechanical crush count, and meeting the following nominal sizes and percents passing will be considered an equivalent Class I material (100% passing 1-inch sieve; 75-95% passing a 3/4-inch sieve; 40-70% passing 1/2-inch sieve; and 0-15% passing

Class II Coarse sands and gravel-sand mixtures with a maximum particle size of 40-millimeters (1½-inches), including variously graded sands and gravels containing small percentages of fine, generally granular and non-cohesive, either wet or dry. Soil types GW, GP, SW and SP are included in this class (INDOT Classification for Structural Backfill)

Class III This class is defined as fine sand and clay gravels, including fine sands, sand-clay mixtures and gravel-clay mixtures. Soil types GM, GC, SM and SC (ASTM D2487) are included in this class. These materials will not be accepted as pipe bedding. Class IV Silt, silty clays and clays, including organic-clays and silts of medium to high plasticity and liquid limits. Soil types MH, ML, CH and CL (ASTM D2487) are included in this class. These materials will not be accepted as pipe bedding.

Flowable Fill Unconfined Compressive Strength (28 day) 50 - 150 psi Flow Test - Diameter of Spread > 8 inches ± 1"

1) Design - Mix design must be submitted to and approved by the design shall include a list of all ingredients, the source of all materials, the gradation of all aggregates, the names of all admixtures and dosage rates, and the batch weights. Except for adjustments to compensate for routine moisture fluctuations, minor mix design changes after the trial batch verification shall be documented and justified prior to implementation by the Contractor. A change in the source of materials, or addition or deletion of admixtures or cementious materials will require the mix design to be resubmitted for approval. The Contractor may be required to provide test data from a laboratory inspected by the Cement and Concrete Reference Laboratory, and approved by Town, which shows that the proposed mix design is in accordance with the requirements listed in this specification.

2) Flow Test - The test for flow shall consist of filling a three (3) inch diameter by six (6) inch high open ended cylinder placed upon a smooth, nonporous, level surface to the top with flowable fill. The cylinder shall be pulled straight up within 5 seconds. The spread of the fill shall be measured. The minimum diameter of the spread shall be eight (8) inches. There shall be no noticeable segregation and the material spread shall be at least 8" diameter

3) Placement - The mixture shall be discharged from mixing equipment by a reasonable means into the space to be filled. The flowable fill shall be brought up uniformly to the fill line. Each filling stage shall be as continuous as is practicable. Concrete may be placed on the flowable fill as soon as bleeding water has subsided. All pavements shall be placed according to flowable fill manufacturer's recommendations.

4) Limitations - Flowable fill shall be protected from freezing until the material has stiffened and bleeding water has subsided. As the temperature nears freezing, additional curing time may be

C. Backfill of Trench Excavations for Pipes and Conduits Bedding and Backfill materials samples shall be submitted to the County prior to start of construction. Shall be in accordance with Indiana Standard Spec. 211.01-211.10 and drawing 715-BKFL-01.

D. Bedding

1. Plastic (PVC, HDPE) Pipe

Plastic pipe conduits (PVC and HDPE) shall be provided with No. 8 crushed stone or approved Class I granular bedding material shovel sliced or otherwise carefully placed and "walked" or hand tamped into place from 4- to 8-inches (based in the diameter of the pipe) below the pipe barrel, to a minimum of 12-inches above the crown of the pipe.

Bedding and initial backfill material shall be hand placed around the haunch and sides of the plastic pipe, to ensure proper compaction filling of all voids.

Bedding shall be placed in 6-inch to 8-inch balanced lifts.

2. Ductile Iron Pipe (DI)

Ductile Iron Pipe (DI) conduits shall be provided with Class I or Class II granular bedding material. Class I material shall be shovel sliced or otherwise carefully placed and "walked" or hand tamped into place from 3- to 6-inches (based upon pipe diameter) below the pipe barrel, to 1/6th the outside pipe diameter (Bc). Class II material shall be compacted to 90% Standard Proctor Density as a minimum, except where the edge of the pipe trench is located within the pavement zone as specified herein, where Class II material shall be compacted to 95% Standard Proctor Density.

E. Backfill Around Pipe Do not backfill trenches until all piping and utilities have been inspected and until the piping system, as installed, conforms to the requirements as detailed in these Standards and are approved by the Town. Backfill all trenches within State Highway Right-of-Way in accordance with Indiana Department of Transportation Specifications. Backfill trenches in rights-of-way in accordance with the requirements of public authority having jurisdiction.

Initial backfill material shall be hand placed around the haunch and sides of the Plastic Pipe (PVC or HDPE) to ensure proper compaction filling of all voids. Initial backfill shall be placed in 6-inch to 8-inch balanced lifts. Backfill trenches for mainline sewers under paved roads, curbs, and gutters with flowable fill. Prepare upper portion of the trench for surface restoration or pavement replacement.

Compacted granular fill shall be used for backfilling trenches in the following areas: (1) for sewer laterals under paved roads, curbs, and gutters to 5 feet from the edge of pavement or backside of curb; (2) for mainline sewers and laterals under paved drives, paved alleys, and sidewalks within 5 feet from the edge of pavement or backside of curb; and (3) for mainline sewers and laterals not under paved roads but within 5 feet of the edge of pavement. Place backfill in 8-inch layers and compact to 95% Standard Proctor Density. Prepare upper portion of the trench for surface restoration or pavement replacement.

Backfill trenches at unpaved driveways and alleys with suitable excavated material up to the last 12-inches which shall be the same material as the original surface. Place backfill in 8-inch layers and compact to 95% Standard Proctor Density.

Backfill trenches under sidewalks greater than 5-feet from roadways with suitable excavated material placed in 8-inch layers and compacted to 95% Standard Proctor Density.

Backfill trenches in areas not requiring flowable or granular fill with suitable excavated material compacted to produce an adequate foundation for seeding. The top 4-inches of backfill shall not contain stones or other objects larger than 1-inch in maximum dimension. Mound backfill above the finished grade to allow for settlement. Place 6-inches of clean topsoil over the area to be seeded. Grade area to be restored settlement and immediately before restoration.

Maintain backfilled trenches in a smooth and uniform condition until paving or seeding operations are completed. Contractor shall refill and restore to the original grade any settlement in the backfill which takes place within the 1-year warranty period at no additional cost to the Town. For all areas requiring compacted granular backfill to meet 95% Standard Proctor Density, perform compaction tests in accordance with the standard specifications of the Indiana Department of Transportation. The Contractor shall be responsible for payment of all compaction tests.

8.04 Installation of Sewers

A. General The Contractor shall provide all tools, labor and equipment necessary for the safe and expeditious installation of all sanitary sewers, manholes, and appurtenances.

Inspect sewer pipe, manhole sections and appurtenances prior to installation and promptly remove damaged or unsuitable materials with new and unused materials.

B. Installation of Sewers

Sewer pipe shall be laid uniformly to line and grade so that the finished sewer will present a uniform bore.

The Contractor, at his own expense, shall set line and grade by means of laser beam and target for alignment and grade. Sewer pipe shall be laid progressively upgrade with bell upstream in a manner to form close, concentric joints with smooth bottom inverts. After the joint is made, sufficient bedding material shall be placed along each side of the pipe to prevent conditions that might tend to move the pipe off line or grade.

Installed piping systems shall be temporarily plugged at the end of each day's work, or other interruption of progress on a given line. Plugging shall be installed in a manner satisfactory to the Town, and it shall be adequate to prevent entry of animals into the pipe or the entrance or insertion of deleterious materials.

A mechanical plug shall be installed at the end of all sewer stubs for future sewer extensions.

8.05 Installation of Force Mains A. General

The Contractor shall provide all tools, labor and equipment necessary for the safe and expeditious installation of all force mains and appurtenances. Inspect force main pipe and appurtenances prior to installation and promptly remove damaged or unsuitable materials with new and unused materials.

B. Installation of Force Mains

Force mains shall be laid uniformly to line and grade so that the finished sewer will present a uniform bore. The Contractor, at his own expense, shall set force main alignment and grade for all sewers. HDPE force mains shall be butt fusion bonded at grade level and lowered into the trench using nylon slings to avoid damage to the pipe. Chains or cable type chokers must be avoided when lifting used sections of pipe. Sewer pipe shall be laid progressively upgrade with bell upstream in a manner to form close, concentric joints with smooth bottom inverts.

After the joint is made, sufficient bedding material shall be placed along each side of the pipe to prevent conditions that might tend to move the pipe off line or grade.

Installed piping systems shall be temporarily plugged at the end of each day's work, or other interruption of progress on a given line. Plugging shall be installed in a manner satisfactory to the Town, and it shall be adequate to prevent entry of animals into the pipe or the entrance or insertion of deleterious materials.

After the main is installed, sufficient bedding material shall be placed along each side of the pipe to prevent conditions that might tend to move the pipe off line or grade. Bedding of #8 crushed stone shall be provided from 4-inches below the pipe to 12-inches above the crown of the pipe. Locate tape and wire shall be installed above all force mains within 2-feet of the finished grade. Tracer wire shall be installed above PVC force mains. It is the responsibility of the Contractor to protect these markers and to verify that all force mains have been properly marked.

8.06 Structure Installation

Manhole excavation shall be kept free from water during construction. Space excavated below the depth required for the manhole base shall be refilled with compacted sand or crushed stone at the Contractor's expense. Manhole floors shall be formed with concrete to a depth of ½ the incoming sewer size with the floor sloped to the sewer invert. Floors for "straight through" manholes shall be formed by laying the pipe straight through the manhole, pouring the concrete and then cutting out the top half of the pipe. Floors shall be constructed as the manhole is being built and this thickness is in addition to the 8-inch base required. Precast concrete risers and adjusting rings shall be used in such combination that the top of the eccentric cone section, when installed, will be at a proper elevation for the manhole frame. Concrete adjusting rings shall be allowed up to a maximum of 12-inches height adjustment. Manholes needing more than 12-inches adjustment shall have a concrete riser section installed to the proper elevation.

Manhole frames shall be brought to grade, centered and embedded in a mastic bonding course. Manhole frames shall be bolted or anchored through adjusting rings and into the cone section of the manhole using stainless steel all-thread or expansion bolts.

Steps shall be built into all manholes 4-feet in depth or greater and shall begin 8-inches below the bottom of the chimney. Steps shall be installed at 16-inches on

Precast concrete risers and cone sections shall be installed so that the axis of the manhole is vertical. Gaskets for the riser joints shall be installed in accordance with the manufacturer's recommendations. Riser joints shall be wrapped with external joint seals in accordance with manufacturer's

Unless otherwise indicated, castings for manholes shall be set at finish grade level. This Contractor shall be responsible for adjusting the casting to the satisfaction of the Town at the Contractor's expense. The Contractor shall remove all debris and excess soil from the manholes after construction and prior to flushing the sewer pipes, to the satisfaction of the Town.

8.07 Pavement Installation Pavements shall be installed as directed under the Town's Subdivision Control

Ordinance.

8.08 Installation of Building Sewers

(Laterals)

A. General Laterals shall be installed at a normal slope of 1/4-inch per foot. Minimum slope shall be 1/8-inch per foot. A mechanical plug shall be installed at the end of each lateral. It shall be the responsibility of the Contractor to install the lateral at a depth sufficient to allow each customer to connect to the end of the lateral.

A piece of resteel shall be installed at the end of each sewer lateral for marking. It shall be the responsibility of the Contractor to protect these markers and to verify that all laterals have been properly marked.

B. Connection to Sanitary Sewers

Connections to new sanitary sewers shall be made only at the manufactured fitting. Connections to existing sanitary sewers shall b made at existing manufactured fittings, as shown on the approved record drawings. In the event that no manufactured fitting exists in an existing sanitary sewer, and if approved by the Town, a service connection may be made using a saddle connection or by cutting into the sewer and installing a manufactured fitting using Fernco couplings.

C. Connections to Sanitary Manholes Connections to sanitary manholes shall not be made without prior

approval of the Town. Building sewers shall connect to the manhole, when approved, at the manhole invert. Under these circumstances, the manhole base shall be pre-formed with a gasketed connection for the lateral line. The invert shall also be formed at the flow line to accept the lateral sewer connection. No inside drop connections shall be allowed without the prior written approval of the Town.

D. Bedding and Backfill

Bedding and Backfill shall be required per the specifications for PVC flexible pipe included in these Standards.

E. Laying of PVC Building Sewer The point of commencement for laying of the building sewer pipe shall be at the connection to the sanitary sewer and shall be laid with the bell end

Whenever a service lateral has to go under street pavement to get to the building to be served, an additional 6-inch cleanout shall be installed at the edge of the right-of-way, on the building private property. Tracer wire and location tape shall be installed from the connection to the

mainline sewer to the edge of the right-of-way or easement.

SECTION 10 INSPECTION, TESTING AND ACCEPTANCE OF SANITARY SEWERS

10.01 General

pointing upgrade.

This section describes the minimum requirements and general procedures for the inspection, testing and acceptance of systems dedicated to the Town of Cumberland.

10.02 Inspection

The Developer's Contractor shall test all manholes, gravity sewers, laterals and force mains installed. New gravity sewers shall also be

All necessary equipment and instrumentation required for proper completion of the flushing and testing of the manholes and piping systems shall be provided by the Contractor. Source and quality of water, test procedures, and disposal of water shall be approved by the Town.

All tests shall be made in the presence of the Town or its Agent. Preliminary tests made by the Contractor without being observed by the Town or its Agent will not be accepted. Notify the Town at least 48 hours before any work is to be inspected or tested.

All defects in piping systems shall be repaired and/or replaced and retested until they are found to be acceptable to the Town. Repairs shall

be made to the standard of quality specified for the entire system. Sections of the system may be tested separately, but any defect which may develop in a section previously tested and accepted shall be promptly corrected and retested.

All manholes and piping systems shall be tested in accordance with these test methods in addition to any test required by the Indiana Department of Environment Management, State or Local plumbing codes and/or building authorities.

10.03 Gravity Sewer Testing

A. General After backfill has been placed, the Town will visually inspect all gravity flow lines to check alignment and grade. All obstructions shall be removed. Any sewer in which the direct light of a lamp cannot be viewed in either direction between adjacent manholes shall be considered unsatisfactory and shall be repaired by the Contractor immediately. Unless otherwise directed by the Town, all underground sewer system piping for gravity flow shall be subjected to an air test rather than an infiltration or exfiltration tests, however, infiltration and exfiltration test methods may be requested by the Town during construction. When leakage occurs in excess of the specified limits, defective pipe or joints shall be located and repaired. The Contractor shall remove and reconstruct, along with retesting, as much of the original work as necessary to obtain a sewer test within the allowable leakage limits.

B. Air Tests

Air tests shall be per ASTM F1417, standard test method for installation acceptance of plastic gravity sewer lines using low pressure air, latest revision. The sewer line to be tested shall be tested in increments between manholes. The line shall be sealed at each end. The seal at one end shall have an orifice through which to pass air into the pipe. An air supply shall be connected to the orifice at one end of the line. The air supply line shall contain an on/off gas valve and a pressure gauge having a range of

psi. The gauge shall have minimum divisions of 0.10 psi and shall have an accuracy of ±0.04 psi.

The pipe line under test shall be pressurized to 4 psig. The line shall be allowed to stabilize between 4 psig and 3.5 psig for a period of no less than 5 minutes. If

necessary, air shall be added to the line to maintain the pressure above 3.5 psig. After the stabilization period, the gas valve shall be closed. When the line pressure drops to 3.5 psig, commence timing with a stop watch. The stop watch shall be allowed to run until such time as the line pressure drops to 2.5 psig. If the test time is greater than the allowable time for 1.0 psig pressure drop, the test section will have

passed the pressure test.

Allowable times are shown in the following table.

IADEL I		IDICATED			LD I OI		JIO ! ! LEC	700 N. D	101 101	VOIZE A	1D LLI1O
Pipe Diameter (in.)	Minimum Time (min:sec)	Length for Minimum Time (fL)	Time for Longer Length (sec)	Specification Time (min:sec) for Length L Shown Below in feet							
			` '	100	150	200	250	300	350	400	450
4	3:46	597	0.380 L	3:46	3:46	3:46	3:46	3:46	3:46	3:46	3:46
6	5:40	398	0.854 L	5:40	5:40	5:40	5:40	5:40	5:40	5:42	6:24
8	7:34	298	1.520 L	7:34	7:34	7:34	7:34	7:36	8:52	10:08	11:24
10	9:28	239	2.374 L	9:26	9:26	9:26	9:53	11:52	13:51	15:49	17:48
12	11:20	199	3.418 L	11:20	11:20	11:24	14:15	17:05	19:56	22:47	25:38
15	14:10	159	5.342 L	14:10	14:10	17:48	22:15	26:42	31:09	35:36	40:04
18	17:00	133	7.692 L	17:00	19:13	25:38	32:03	38:27	44:52	51:16	57:41
21	19:50	114	10.470 L	19:50	26:10	34:54	43:37	52:21	61:00	69:48	78:31
24	22:40	99	13.674 L	22:47	34:11	45:34	56:58	66:22	79:46	91:10	102:33
27	25:30	88	17.306 L	28:51	43:16	57:41	72:07	86:32	100:57	115:22	129:48
30	26:20	80	21.366 L	35:37	53:25	71:13	69:02	106:50	124:38	142:26	160:15
33	31:10	72	25.852 L	43:05	64:38	86:10	107:43	129:16	150:43	172:21	193:53
36	34:00	66	30.768 L	51:17	76:55	102:34	128:12	153:50	179:29	205:07	230:46
							. =				

If the time lapse is greater than that specified, the section undergoing tests shall have passed. If the time is less than that specified, the line has not passed the test and the Contractor shall be required to make all repairs and retests. If the pipe line to be tested is beneath the ground water level, the test pressure shall be increased 0.433 psi for each foot the ground water level is above the crown of the pipe.

The Contractor shall furnish all equipment and personnel required to make all tests including pipe stoppers, air compressor, air storage tank, pressure regulating valves, pressure gauges, stopwatch, etc. The Contractor shall take all precautions necessary, including blocking of stoppers or plugs, to protect the safety of property and personnel.

C. Infiltration Tests

When the groundwater level is 4-feet or more above the top of the sewer, the infiltration test will consist of sealing off a length of sewer and measuring the depth of flow over a measuring weir, or by pumping the infiltrated water into containers for measurement. Tests shall be conducted for a minimum of 4-hours. Infiltration leakage shall not exceed 200 gallons per 24 hours per inch diameter, per mile of

D. Exfiltration Tests

When the groundwater level is below a point 4-feet above the top of the pipe, the exfiltration test shall consist of isolating the particular section and filling the water to a point 4-feet above the ground water level in the upper manhole and allowing it to stand not less than 4-hours. The section shall then be refilled with water up to the original point and after 2 hours the drop in water surface shall be measured. The computed leakage shall not Exceed 200 gallons per inch diameter, per 24 hours, per mile of sewer.

E. Deflection Tests

Deflection tests shall be performed on all flexible pipes after the final backfill has been in place at least 30 days. No pipe shall exceed a vertical deflection of 5%. Deflection testing shall be performed using a mandrel pulled by hand. Ductile iron pipe is considered non-flexible and will not require a deflection test.

10.04 Gravity Sewer Television Inspection

A. After new gravity sanitary sewers have been completed and all sewers have passed the required testing, new segments shall be internally inspect, via closed circuit television (CCTV) inspection, by the Developer. These inspections shall be recorded on VHS color video tape or CD/DVD, including a narrative noting:

1. Date and time of day;

2. Sewer segment number "from manhole to manhole"; 3. Locations of service connections (laterals) into sewer; 4. If detected, the location of obstructions, structural defects, leakage or evidence thereof, and other abnormalities with respect to the sewer condition and distance in feet from the

B. The video shall visually display date, pipe section number (manhole number) and distance from upstream manhole (accuracy of 2 feet±). Where an obstruction is encountered and a reverse set up is required, the distance shall be written and verbally noted on the video as to from which manhole measurements are being made. Video case shall display the same information as indicated above plus date and crew ID number. Videos of all sections shall be provided to the Town along with the respective television inspection field logs. TV field logs shall legibly show the location of each point of significance in relation to an identified manhole, including

service connections (laterals).

upstream manhole centerline.

C. Video shall be a continuous image of not less than ninety percent (90%) of the internal pipe surface at all times. Maximum acceptable speed of camera through sewer shall be thirty (30) feet per minute. Lighting system shall be adequate for quality pictures. A reflection in front of the camera may be required to enhance

D. If any obstruction in the sewer segment prohibits the passage of the television camera, Contractor shall inspect the remainder of the sewer segments by making a reverse set up at the next down stream manhole. E. All obstructions in the sewer segment that prohibit passage of the television camera shall be immediately reported to the Town by Contractor referencing location and nature of the obstruction. F. The video camera shall be equipped with remote control devices to adjust the light intensity and a minimum one thousand (1,000) feet of continuous cable shall be provided. The camera shall be able to transmit a continuous image to the television monitor as it is being pulled through the sewer segments. G. Developer shall submit the original video tapes or CD/DVD, along with corresponding written televising log sheets, to the Town for review at the

conclusion of the project and prior to acceptance of the sewers.

10.06 Manhole Vacuum Testing

A vacuum test shall be conducted by the Contractor on all manholes to ensure water tightness and manhole integrity. The equipment required to conduct a vacuum test on manholes includes inflatable pipe plugs, test head, vacuum pump, flexible air hose and a vacuum gage. The test equipment shall be capable of drawing a vacuum of 10-inches Hg. The equipment shall be designed specifically for the purpose of testing manholes and shall be as manufactured by P.A. Glazier, Inc., Worchester, Massachusetts, 10002 or as approved by the Town. The procedure for conducting an air test on manholes shall be in accordance with the following procedure:

 Each manhole shall be tested immediately after assembly and prior to setting the casting or backfilling around the structure. • All lift holes shall be plugged with non-shrink grout. · All pipes entering the manhole shall be securely plugged and adequately braced against the inside of the manhole to prevent being drawn out of the pipe. • The test head shall be placed on the inside of the cone section and sealed with an inflatable seal.

• A vacuum of 10-inches of mercury (Hg) shall be drawn before the vacuum pump is shut off. With the valves closed, the time shall be measure for the vacuum to drop to 9-inches. The manhole shall pass if the time is greater than the following:

	Manhole Diameter								
Manhole Depth (ft)	48-in	60-in	72-in						
Doptii (it)	Minimum Test Time, seconds								
8	20	26	33						
10	25	33	41						
12	30	39	49						
14	35	46	57						
16	40	52	67						
18	45	59	73						
20	50	65	81						
22	55	72	89						
24	59	78	97						
26	64	85	105						
28	69	91	113						
30	74	98	121						

SPECIFICATIONS AND DETAILS NOTE

- TOWN OF CUMBERLAND DETAILS AND SPECIFICATIONS SUPERCEDE ANY CONFLICTING INFORMATION BETWEEN THE GENERAL DETAILS (SHEETS C10.0 - C10.1) AND THE TOWN OF CUMBERLAND (SHEETS C10.2 - C10.4).



